Knox Hockey Club Inc. | PO Box 5181, Studfield, VIC, 3152

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1 April 2021

By email

Tanya Scicluna Director – Connected Communities Knox City Council

Dear Tanya

Knox Hockey Club – New Facility

The Knox Hockey Club is pleased to provide this further letter of support in relation to the ongoing efforts of Knox City Council to come to a final resolution in relation to the future siting of a home for the Club, and the construction of a facility commencing in the FY 2021-22.

Work undertaken by the Club

Over recent years, working together with Council officers on this project, the Knox Hockey Club has:

- Maintained a strong working relationship with The Knox School to extend the Club's tenancy as long as possible;
- Maintained active communications with Hockey Victoria in relation to the prospect of requiring a new location, to facilitate engagement from HV and assistance in relation to any transitional issues which might arise;
- Maintained a strong financial position in difficult circumstances in order to ensure the Club is able to provide their financial contribution to a new hockey pitch development, including membership expansion and other engagement with multiple State and Council-based grant opportunities;
- Worked in co-operation with officers in relation to each site option that has been brought forward for consideration, including providing practical and operational feedback at each opportunity;
- Engaged with other local stakeholders around proposed pitch locations where appropriate (e.g., Maroondah City Council, Scoresby Secondary College) to build relationships; and
- Reached out to other hockey Clubs in Melbourne which share direct residential interfaces with their hockey ground to obtain information and precedents on community management arrangements.

Proposed sites and further investigation

At this point in particular, the Knox Hockey Club would like to provide support for the proposal for further investigation of potential sites to be completed by Council, including sites at Wantirna Reserve (Wantirna), Gilbert Park (Knoxfield), and JW Manson Reserve (Wantirna).

The Club is prepared to provide whatever reasonable support and resources are required in order to ensure such investigation is progressed as immediately as possible. This is consistent with correspondence that the Club provided to Council in March 2021 confirming the continued need for a new facility and the Club's desire to focus on the use of Wantirna Reserve as their preferred outcome.

Knox Hockey Club believes that each of these sites should be investigated with priority given their potential significant upside in comparison to Benedikt Reserve. The current proposal of Benedikt Reserve is of concern to our Club given that the minimum operational requirements of a hockey club operating on a single pitch involve on-field activity extending to 10pm on weeknights to facilitate training and mid-week competitions, and 8pm on weekends to facilitate weekend competition when the Club is playing at home. Whilst competition hours change from week to week during the season (April – October), protection of these hours are critical to ensure that the Club remains viable with a single pitch solution. These hours, together with the significant change involved with developing the existing open space are

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likely to impose substantial change upon local residents that has to date attracted substantial grassroots community opposition.

Notwithstanding this, Knox Hockey Club will support Benedikt Reserve for the delivery of a single hockey pitch should it be the only option available for delivering a pitch during 2022.

Knox Hockey Club appreciates the work being undertaken by Council in relation to this project, and is committed to continuing to work and engage with Knox City Council, Knox Councillors, Hockey Victoria, local resident groups and local MPs in order to progress successful relocation in 2022.

Impact of delays – temporary solution

The Club notes that delays as a result of ongoing consultation, whilst potentially necessary, do place the future of the Club and hockey in Knox at risk in 2022. With the Club's existing home at The Knox School becoming unavailable from November 2021 onwards, without certainty in relation to siting and construction timelines there is now substantial risk of the Club starting the 2022 season without a home pitch to facilitate training and competition.

To mitigate against the risk of being without a home during 2022, the Club is actively exploring temporary alterative venues in conjunction with Hockey Victoria, which would allow the Club to continue operating in the short term. Any alternative location will need to be carefully chosen in order to provide a level of certainty to and consistency for our members, noting that it cannot be located within any suburb in the Knox municipality.

In absence of a plan to open a new pitch during 2022, the Club is liable to lose a significant number of playing members. The members the Club is at highest risk of losing include:

- Highly skilled young developing players with regional and state level experience who are looking for a hockey Club with a certain future (players in this category have already started to be lost in recent months), and
- Junior and lower grade senior players who are not prepared to travel significant distances to train and play.

Whilst membership for Knox Hockey Club increased marginally ahead of the 2020 season, membership numbers remain below trend because of:

- Limited and deteriorating facilities at the Knox School (including no pavilion access and poor standard of playing surface; and
- lack of certainty about the Club's future due to the planned closure of the school pitch.

As such, it is essential that this project remains a priority for Council and for the Club in coming weeks and months to ensure the most appropriate outcome is achieved within the delivery timelines required.

We look forward to continuing to engage with Council in relation to this matter and working collaboratively to identify and progress a satisfactory solution for hockey in the Knox municipality.

Kind regards

Stephanie Mann

Stephanie Mann President – Knox Hockey Club 0458 602 860 (M) president@knoxhockeyclub.com.au

Attachment 9.3.2





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INTRODUCTION

The Outer East Region includes four of the seven LGA's that make up the broader Melbourne East Region. The Cities of Maroondah, Knox and Monash, along with the Shire of Yarra Ranges, have partnered on this project to understand the current and future needs for hockey in the Outer East.

The broader Melbourne East Region provides a strong market for hockey with over 3,000 registered participants contributing approximately 15% of the total number of players across the state. High participation rates combined with gaps in facility provision and an additional 100,000 people expected in Melbourne's East over the next 10 years, supports a more strategic regional solution to address the future needs of hockey.

Hockey in the Outer East consists of four clubs (one premier league), four synthetic pitches across four venue and a total of 1,263 players connected to clubs within the study area. To guide the future planning and development of hockey in the Outer East this report delivers the following outcomes:

- A comprehensive participation analysis to understand the existing hockey climate in Melbourne's Outer East.
- An overview of current issues and challenges facing hockey participation and sustainability.
- Key findings from stakeholder consultation and priorities for the future hockey provision development.
- Existing facility access analysis including travel times for participants.
- · An overview of existing and future programming.
- Investigation of proposed future sites for the development of new facilities.
- A prioritised implementation plan for the next 10 years, aligned with stakeholder responsibilities.
- An overview of development cost, viability and preferred management models.

A catalyst for this project (although not the sole driver) is the pending closure of the Knox Hockey Club pitch, currently located at the Knox School, until October 2021. Knox Council will use the findings from this study to inform decision making to support the Knox Hockey Club in finding a new home.

The model of community hockey facilities being located within schools and universities is a key issue in the Outer East that will need to be addressed to support club sustainability. In addition to the Knox issue mentioned above, two of the remaining three hockey venues are located on land not owned by local government. Yarra Valley Grammar and Monash University (Clayton Campus) provide facilities for the Croydon Hockey Club and Monash University Hockey Club respectively. There is uncertainty around the long term future for hockey at both of these sites.

Consultation with clubs within the study area and across the broader Melbourne East Region identified the following key themes:

- Lack of hockey infrastructure to support the future growth of hockey.
- · Limited access and development opportunities at existing facilities.
- Replacement pitch required for Knox and additional pitches required to meet current and future demand.

The continued growth of Hookin2hockey and junior hockey, and the emergence of new social and modified programs such as Hockey Sixers and J-Ball, will drive future facility development opportunities. The delivery of new hockey facilities in the Outer East should be guided by the principle of shared use to accommodate the trends in unstructured participation being offered at flexible times.

This report provides information on existing facility classification levels identified by Hockey Victoria which have been used to inform future facility recommendations that are applicable to the Outer East Region.

DEFINITIONS AND ABBREVIATIONS

The following definitions and abbreviated terms are used within this document.

ABBREVIATION	DEFINITION
LGA	Local Government Authority. May also be referred to as Council.
Melbourne East Region	Refers the seven LGAs that make up the Melbourne East Region - The Cities of Boroondara, Manningham, Whitehorse, Maroondah, Knox and Monash and the Shire of Yarra Ranges.
Melbourne Outer East Region	The study area. Refers to the four LGAs that make up the Melbourne Outer East Region - The Cities of Maroondah, Knox and Monash and the Shire of Yarra Ranges.
	For this project the term 'Regional Hockey Facility' is used in the context of providing a regional solution for hockey within the study area and does not align with the true definition of a regional facility.
Regional Hockey Facility	For reference only, page 28 of this document provides the range of hockey facility classification levels from State to Club level as prescribed by Hockey Victoria.
Single or purpose built hockey pitch	For this project the term 'single or purpose built hockey pitch' refers to a pitch that is used for hockey only.
Multiuse / multipurpose hockey pitch	For this project the term 'multiuse' or 'multipurpose' hockey pitch refers to a pitch that is being used or will be used for other activities by compatible sports such as soccer, tennis, touch, lacrosse etc. These pitches are generally multi-lined but can still host competition games and events/tournaments.
Local club venue	For this project the term 'local club venue' refers to existing venues where local clubs are based or are using for training purposes.
Competition venue	For this project the term 'competition venue' refers to full size compliant hockey pitches that are being used for competition purposes.

HOCKEY IN THE OUTER EAST

Hockey is a popular sport in Victoria with 21,913 players (19,273 playing members and 2,640 Hookin2hockey players). With 1,263 registered players the Melbourne's Outer East Region contributes approximately 6% of total participants across the state.

Hockey Victoria is projecting significant growth within the sport in the next 5-10 years through the continued increase in junior player numbers and the success of social formats. Hockey Victoria is aiming for 25,000 members by 2023.

Over the past three years club hockey members based in Melbourne's Outer East has increased by 9% from 1,155 in 2015 to 1,263 in 2017. All clubs, with the exception of Monash University Hockey Club, have increased their membership numbers since 2014/15. The Croydon Hockey Club has experienced the greatest increase growing by over 200% to 179 players in 2017. In 2014 the club only had 3 senior teams and they now field senior women's and mixed junior teams.

Three clubs (Monash, Knox and Waverley) are located within Hockey Victoria's East Metropolitan Zone. Croydon Hockey Club is in the North East Metropolitan Zone. Hockey Victoria's 2015 Strategic Facilities Master Plan identified the following recommendations for the Outer East Region. It is important to note that the loss of the Knox Hockey pitch was unknown at the time these recommendations were developed.

- Development of a second pitch at Ashwood Reserve, Monash
- Renewal of the Monash University pitch.

Based on a population of 635,0019 the current pitch to population ratio for the Outer East Region is 1:158,755. This easily exceeds the industry pitch to population ratio guideline of 1:100,000 and indicates that additional pitches are required. The current pitch to player ratio is 1:316.

In comparison hockey in the G21 (Geelong) Region has a pitch to population ratio of 1:46,653. The G21 Region has a similar number of players but far less population and a greater number of pitches.

Melbourne Outer East Regional Hockey Feasibility Study | Final Report | November 2018



Hockey players in the Outer East

- 9% increase in club membership from 2015 2017
- 620 registered adult participants (49%)
- 460 junior participants (36%)
- 183 Hookin2Hockey participants (14%)
- 40% female / 60% male gender breakdown
- Average player age is 22 years

PROJECT DEVELOPMENT

The Melbourne Outer East Hockey Feasibility Study has been delivered in four key stages and is due to be finalised and adopted by project partners by the end of 2018. The diagram below outlines project timelines and associated key tasks delivered during each stage in accordance with the project scope. The project is currently in Stage 3 with the draft report being considered by the Project Control Group.





STRATEGIC PLANNING FOR HOCKEY

Following is a summary of key strategic documents that will impact the future planning and development of hockey facilities in the Outer East Region.

HOCKEY VICTORIA PLAYER AND STAKEHOLDER RESEARCH (2018)

State-wide hockey research undertaken by Latrobe University in February 2018. Key highlights include:

- 95% of active players reported that they are likely to be still playing hockey in 12 months time and 86% in 3 years time.
- 84% of current players indicated that they would be very interested in playing at least one new format of hockey.
- 77% of all Victorian players play in metropolitan areas (23% regional).
- 77% of inactive players reported interest in playing a new format of hockey.
- Average age of the current Victorian hockey player is 38 years.
- On average, the Victorian hockey player has been playing the game for 25 years.
- 59% of current players have other family members playing hockey.
- 53% of respondents plays multiple times per week, and 39% train multiple times per week.
- 40% of participants identified a need for more localised hockey competitions (decreasing travel time).
- 34% of participants highlighted the need for hockey to be offered at different times of the year.
- 30% identified a desire to play a mixed gender format of the game.
- The top 3 reasons for inactive players ceasing hockey participation included family commitments, work / school commitments and travel for match play was too far.

HOCKEY VICTORIA STRATEGIC FACILITIES MASTER PLAN (2014)

A plan developed by Hockey Victoria to address Facilities needs.

Recommendations relevant to Melbourne's Outer East Region included in this Plan are provided below.

It is important to note that the loss of the Knox Hockey pitch was unknown at the time these recommendations were developed.

- Development of a second pitch at Ashwood Reserve.
- Development at a second pitch at Elgar Park.
- Renewal of the Monash University pitch.
- Since 2014, the Monash University pitch upgrade has been completed, and concept designs have been developed for the second pitch at Ashwood Reserve, with further planning required before further project development.

MELBOURNE EAST SPORT AND RECREATION STRATEGY (2016)

No hockey specific recommendations were outlined in this Strategy, although demand for hockey was identified as a high priority.

The Strategy highlighted the important role regional sporting facilities play in contributing to the health and well being of communities as they generally serve a broad catchment and cater for a diverse range of activities.

The Strategy investigated the issues and opportunities impacting the planning and provision of regional level facilities and is a key resource that should be considered to guide the future planning and development of regional level facilities.

PARTICIPATION ASSESSMENT

The table below provides an overview of Melbourne Outer East Hockey Club membership trends for the previous three years (2015 - 2017).

- Overall, hockey player numbers in Melbourne's Outer East Region experienced an increase of +108 participants (9%) from 2015 to 2017.
- After the reformation of Croydon Hockey Club in 2015, their membership growth has experienced growth of over 200% (+121 members).
- Knox Hockey Club and Waverley Hockey Club membership has remained relatively steady across the three year period with modest participation increases.

CLUB	2015	2016	2017	TOTAL	Member Change (+ / -)	% growth
Croydon Hockey Club	58	109	179	346	+121	209%
Knox Hockey Club	314	338	328	980	+14	4%
Monash University Hockey Club	258	265	216	739	- 42	-16%
Waverley Hockey Club	525	556	540	1621	+15	3%
TOTAL	1155	1268	1263	3686	+108	9%

Hockey members at clubs within the study area.

The map on the following page provides a visual representation of current hockey players living (by post code) and playing (by home club) within the study area.

The heavy red colour indicates the highest areas of hockey membership. Key participation hot spots by post code for hockey in the region are all located within the City of Monash:

- 3149 Mount Waverley (148 hockey players).
- 3147 Ashburton and Ashwood (121 hockey players).
- 3150 Glen Waverley and Wheelers Hill (113 hockey players).

Other relevant participation statistics include:

- There are 1,404 registered hockey players living in the study area (regardless of where they play). Doncaster Hockey Club (136), Camberwell (59) and Greater Dandenong Warriors (42) attract the most players from within the study area.
- 825 out of 1,263 (65%) club hockey players that live and play in the study area. The Waverley Hockey Club (328) have the greatest number of players living in the study area, followed by Knox (258) Croydon (157) and Monash University (82).
- 438 hockey players live outside the study area but are members of clubs inside the study area.
- 372 resident hockey players live in the study area but play at clubs outside the study area.
- There are 3,348 hockey players playing at other clubs within the Melbourne East Region that live either within or outside the study area.



EXISTING FACILITY PROVISION

The four synthetic hockey pitches in the Outer East Region are located in Monash (2), Maroondah (1) and Knox (1). Geographically the existing pitches are well spread across the study area which is enabling convenient access for players.

Currently there is no provision for hockey within the Shire of Yarra Ranges, although the soccer pitch at the Yarra Hills Secondary College in Mooroolbark is used by the Croydon Hockey Club for pre season training.

Existing facility provision across the Outer East is listed below.

ASHWOOD RESERVE

Monash City Council Ashwood Waverley Hockey Club 1 hybrid synthetic grass pitch Lighting provided (200 - 250 lux) New LED lighting to 500 lux due late 2018

MONASH UNIVERSITY

Monash City Council Clayton 1 hybrid synthetic grass pitch Monash University Hockey Club Lighting provided (200 - 250 lux)

THE KNOX SCHOOL

Knox City Council Wantirna South 1 synthetic grass pitch Knox Hockey Club Lighting provided (250 - 500 lux)

YARRA VALLEY GRAMMAR Maroondah City Council Croydon Hills Croydon Hockey Club 1 synthetic grass pitch

Lighting provided (250 lux)

YARRA HILLS SECONDARY SCHOOL (no located on map) 1 synthetic grass pitch

Not hockey specific No home club (overflow venue) No lighting provided



Facilities

HEDLEY HULL FIELD

- MONASH UNIVERSITY CLAYTON CAMP.
- THE KNOX SCHOOL
- YARRA VALLEY GRAMMAR SCHOOL

FACILITY USAGE

An analysis of the current usage at all four hockey pitches within the study area was undertaken to determine if pitches were exceeding recommended usage guidelines.

The hockey pitch at Ashwood Reserve in Monash is the most heavily utilised for both training and competition within the Outer East Region. The pitch is currently used approximately 43 hours per week in-season for training and competition to service the clubs 540 members.

This level of use would be considered excessive in accordance with Hockey Victoria's advice that 36 hours per week should be the maximum use for a single pitch facility.

The next most heavily utilised pitch is Monash University which is used by the Monash University Hockey Club on an average of 32.5 hours per week in-season to service their 216 members. Training hours for this pitch appear lower than other pitches which has not been explained.

The Yarra Valley Grammar School has the lowest usage (less than 10 hours a week on average) and is used by the Croydon Hockey Club which have the lowest membership base. The limited access provided to the club under the current arrangement with the school could also be a factor for the low usage.

The Yarra Hills Secondary College is only being used moderately (less than 10 hours per week) for junior programs and pre season training by the Croydon Hockey Club is not appropriately dimensioned for competition hockey.

The following graphs and tables provide a more detailed breakdown of the current pitch usage (training and competition) at each venue compared to recommended capacity guidelines provided by Hockey Victoria.





Training times

AVERAGE MONTHLY PITCH USAGE

	Competition use	Training schedule	Total weekly use	Capacity hours
Ashwood Reserve	23.3	19.75	43.0	36
Monash University	25.5	7	32.5	36
Knox School	11.3	12.05	23.3	36
Yarra Valley Grammar	9.6	4	13.6	36
Yarra Hills Secondary – preseason and Saturday use only	2	0	2	36

Monthly pitch usage has been sourced from Hockey Victoria fixtures (2018) and club consultation.



Weekly use v capacity

The weekly pitch capacity of 36 hours per week has been provided as a guideline by Hockey Victoria.

Attachment 9.3.2

ACCESS TO FACILITIES

With fewer hockey pitches within reach of the general population compared to football ovals or basketball courts, there is a general acceptance of the need to travel to play hockey. However, the constant demand on time poor parents to drive their children significant distances to play hockey is seen as a key issue to growing the game.

A breakdown of player travel times to their respective clubs within the Outer East Region is provided below. On average approximately 80% of players are travelling up to 20 minutes to access their club facility.

Ashwood Reserve (Waverley Hockey Club)

- 540 members
- 445 (83%) live within a 20 minute drive of the facility

Monash University (Monash University Hockey Club)

- 216 members
- 143 (67%) live within a 20 minute drive of the facility

Yarra Valley Grammar (Croydon Hockey Club)

- 179 members
- 160 (90%) live within a 20 minute drive of the facility

The Knox School (Knox Hockey Club)

- 328 members
- 275 (83%) live within a 20 minute drive of the facility





Travel times to access existing facilities - 10 minute (grey shaded area) and 20 minute (yellow shaded area).

MELBOURNE

OUTER EAST

HOCKEY

CLUBS

STAKEHOLDER CONSULTATION SUMMARY

Detailed responses obtained through the consultation process have been outlined below to provide a greater level of understanding of the current needs, issues and priorities of key hockey stakeholder in the Melbourne East Region.

KNOX HOCKEY CLUB

- The lack of hockey infrastructure across the region is limiting opportunities for the sport.
- Some decline in club membership numbers across the past five years due to the re-formation of the Croydon Hockey Club.
- Visibility and access to the school restricts ability to grow the club and attract new members.
- Annual cost of Hockey Victoria membership has increased significantly putting financial strain on the Club.
- Uncertainty around a future facility due to the clubs lease with the school not being renewed due to the removal of the pitch.

MONASH UNIVERSITY HOCKEY CLUB

- Membership decline in recent years due the single pitch being at capacity. Difficult to schedule training with lack of pitch availability.
- Recent years has seen growth in Under 10's and 12's competition.
- Female friendly change facilities required to support female teams.
- Lack of facilities within the Melbourne East Region is hampering the ability of clubs to grow.

CROYDON HOCKEY CLUB

- Home base at Yarra Valley Grammar School community assume it's a school based rather than a community based club.
- Multiple facility constraints at Yarra Valley Grammar including storage, shelter and poor floodlighting.
- Yarra Valley Grammar is not the clubs preferred long term option due to access constraints and the impact on potential growth. Year to year contract with the school makes it difficult for the club to invest in facility improvements / upgrades.
- Juniors use non hockey field at Yarra Hills Secondary College due to access limitations at Yarra Valley Grammar. There has been significant growth in Hookin2Hockey numbers.
- The Club would like a centralized home ground for both seniors and juniors to continue to grow membership numbers.

WAVERLEY HOCKEY CLUB

- Currently turning away members due to the pitch being at capacity.
- Significant growth being experienced in the U10 and U12 competition.
- Second pitch development to meet current demand is the highest priority.
- Lack of facilities across the Region creates issues for all existing clubs.
- Additional facility provision is required along with female friendly change facilities.

KEW BOX HILL HOCKEY CLUB

- The existing pitch at Elgar Park was recently resurfaced.
- A new pavilion commissioned by the City of Whitehorse was completed in 2017 at a cost of \$3.3m has had a positive effect on participation growth.
- The club is seeking the development of a second pitch at Elgar Park to support growth.

MELBOURNE EAST HOCKEY CLUBS

DONCASTER HOCKEY CLUB

- Gradual membership growth over the past two years. Currently at 590 members.
- Additional pitch needed to meet current and future demand.
- Large catchment of players, including many from Yarra Ranges.
- Yarra Valley Grammar used as overflow ground, but is not ideally located or easily accessed by members.

CAMBERWELL HOCKEY CLUB

- A premier league club.
- The largest club in Victoria with over 1,000 members 50+ teams and continued player growth and development opportunities.
- New pavilion close to being completed that will support greater community use and access.
- Club use Koonung Secondary College as well as Matlock Reserve.

HAWTHORN HOCKEY CLUB

- Membership growth across all age groups in recent years.
- Pitch renewal at Hawthorn Malvern Hockey Centre is the clubs highest priority, followed by pavilion renewal (note the building is not council owned).
- Additional pitches in Melbourne's East are required to meet demand.
- Implementation and support for social hockey formats is required.
- Partnerships with schools should be a focus in the region to assist in growing the game.

CITY OF BOROONDARA

- · Three hockey pitches within the municipality.
- 2016 Sport and Recreation Strategy identified a need to increase hockey provision.
- Council is seeking partnerships with schools to increase access to hockey facilities as there is limited open space opportunities to develop new facilities.
- · Camberwell hockey facility is being redeveloped.

MELBOURNE EAST LGAs

CITY OF MANNINGHAM

- Doncaster Hockey Club is a strong and successful club and is growing.
- Council looking to assist the Club in upgrading their facility at Mullum Mullum Reserve.
- The Mullum Mullum Reserve Management Plan completed by Council in 2014 prioritised increased participation opportunities for all sports utilizing the site.

CITY OF WHITEHORSE

- Current land pressure at Elgar Park to support the development of a second pitch. Will be subject to extensive future business planning and be developed as a multi-use facility.
- The existing pitch supports overflow participation for all Whitehorse based hockey clubs.
- The development of additional hockey facilities within Melbourne East requires high specifications for floodlighting, change facilities and a provision for two fields, with flexibility in a proposed management model.

HOCKEY VICTORIA	 KEY ISSUES Limited opportunity to implement social hockey programs as there is no capacity to access facilities during peak times. Current pitch usage and participation in the region is the highest in the State. There is a need to develop more pitches to grow the game. The pending loss of the Knox Hockey pitch. 	 KEY PRIORITIES Increasing capacity of existing facilities to allow for the implementation of social hockey programs. Stronger partnership with Melbourne Outer East Council's and schools to provide more security for hockey clubs. Social hockey incorporated into any new facility development programming schedule. Planning for the development of a replacement pitch for the Knox Hockey Club is a high priority.
MELBOURNE OUTER EAST EDUCATION FACILITIES	 MONASH UNIVERSITY Hockey pitches and other sport facilities are under pressure due to lack of space at the Clayton Campus. Two existing sports fields have already been converted to student accommodation. There is no space for an additional pitch at Monash University. Priority is to retain the existing high grade pitch that has been used at elite level. 	 THE KNOX SCHOOL The Knox Hockey Club is the only external tenant using the pitch. The pitch was developed 20 years ago and has not seen any upgrades since. The land the pitch currently occupies will be redeveloped to expand school facilities beyond 2021. The school has notified the club and Council that the lease will not be renewed. The club has a pitch replacement (sinking fund) which may be used to contribute to the development of a replacement pitch in the future.

SUMMARY OF KEY FINDINGS AND PRIORITIES

A detailed summary of the current issues and key project drivers for the Melbourne Outer East Regional Hockey Feasibility Study are provided below to understand the needs and requirements of key stakeholders. These themes were strongly presented throughout the project engagement process and have informed the development of strategic recommendations.

PITCHES AT OR NEAR FULL CAPACITY

Current pitch usage was analysed to determine the capacity of existing pitches in the Outer East Region. Usage of Ashwood Reserve field is exceeding capacity, with Monash University and Knox operating at approximately 88% and 66% respectively. A breakdown of pitch usage and capacity is provided later in this report.

LIMITATIONS IN DELIVERING NEW SOCIAL HOCKEY PROGRAMS

High pitch usage during peak times by clubs creates challenges for Hockey Victoria in accessing facilities to run the new social hockey formats and promote the game to new markets. Investigating the use of alternate facilities and surfaces will be required to facilitate social hockey in the region.

HOCKEY FACILITIES IN SCHOOL AND UNIVERSITIES

A range of issues were identified with existing facilities being located on non Council land. The future of the Knox Hockey Club and its 328 members is uncertain with the school notifying the club and Council of its intention not to renew the lease for the hockey pitch after October 2021.

Existing arrangements for pitch access and the future use of facilities at Yarra Valley Grammar and Monash University is also uncertain.

GREATER PARTNERSHIPS WITH THE EDUCATION SECTOR

Hockey aims to strengthening partnerships with the education sector to enable greater access to facilities and increase community programming opportunities.

GAP IN HOCKEY PROVISION IN YARRA RANGES.

With no dedicated hockey facilities in Yarra Ranges and with 176 players travelling out of the Shire to participate, there will be a need to investigate shared facility development opportunities in the medium to long term.

A TWO PITCH HOCKEY FACILITY ASHWOOD RESERVE

The findings from this study supports the recommendation in Hockey Victoria's Strategic Facilities Master Plan to develop a second pitch at Ashwood Reserve. The Waverly Hockey Club has 540 members (8th largest in the State) and is unable to service the needs of existing players due to the limitations of a single pitch facility. Short to medium term planning for an additional pitch will be a focus for Monash Council.

HOCKEY PRESENCE TO MEET A GROWING POPULATION

The current hockey pitch to population ratio within the study area is 1:158,755. This will increase to 1:403,978 with the pending loss off the Knox pitch. As a hockey stronghold the Outer East will require additional pitches to meet the needs of current and future population growth, where there is expected to be an additional 44,065 people over the next ten years.

In summary, the following key focus areas were identified from the research and consultation undertaken during the development of the feasibility study:

- Development of a new facility to replace the pending loss of existing facilities.
- Development of a two pitch facility at Ashwood Reserve to meet club and community demand.
- Strengthen partnership with the education sector to increase pitch access on school and university land and provide longer term security for clubs.
- Future hockey provision in Yarra Ranges Shire will be required to provide opportunities for residents to participate and address the travel barrier.

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OUTER EAST PROFILE AND DEMOGRAPHICS

The Melbourne East Region is one of the fastest growing regions in Metropolitan Melbourne. The Outer East (study area) contributes over half (56%) of the total Melbourne East population. Key population and demographic information sourced from profile.id and forecast.id are highlighted below.

MELBOURNE OUTER EAST DEMOGRAPHIC OVERVIEW

- The Outer East Region contributes 56.36% of the total Melbourne East Region population.
- Current Melbourne Outer East Region population is 635,019 with an additional +44,065 people expected to 2028.
- Overall 28.5% increase in population in the next 10 years.
- Monash population: 196,385 (+10,522 to 2028). Greatest age cohorts 40 - 49 years (12.9%) and 20-24 years (10.1%).
- Knox population: 163,170 (+12,939 to 2028). Relatively even age cohorts. 40 49 years highest representation at 14.1%.
- Yarra Ranges population: 158,068 (+10,609 to 2028). Greatest age cohort also 40 49 years (13.9%).
- Maroondah population: 117,396 (+9,995 to 2028). Relatively even age cohorts. 30 39 years highest representation at 14.7%.

MELBOURNE EAST SPORTING PREFERENCE

Participation in organised sport is highly valued in the Melbourne East Region, with basketball, football and netball amongst the most popular sports. Although these mainstream sports dominate overall participation, hockey has a significant presence in the Region.

PARTICIPATION TRENDS

The demand for hockey and the way it is delivered will continue to grow across the Region. The way in which Australians prefer to participate in sport is changing, with busy lifestyles and time constraints playing a significant role in how sport is being consumed.

Participation in unstructured and casual forms of hockey will continue to grow, only further highlighting the need to accommodate social formats of hockey in the Melbourne Outer East Region.

KEY GROWTH AREAS

The map presented on the following page provides the forecast population growth in the Outer East Region between 2017 and 2027.

Key growth areas within the study area over the next ten years include Lilydale, Ringwood and Croydon Hills. These suburbs combined will contribute an additional 12,500 people out of a total forecast population growth of 44,065.



Map of forecast population projections in the study area.

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PROJECTING DEMAND

Applying the forecast population growth to the current penetration rate for hockey in the Outer East Region is one way of projecting the number of hockey players in the future.

The study area penetration rate of 0.19% (one hockey player for every 503 residents) has been calculated by dividing the total number of hockey players at the four clubs (1,263) by the study area population (635,019), multiplied by 100. When this penetration rate is applied to the forecast population it shows an additional 95 new hockey players to 2028.

The following table provides a breakdown of potential future player growth by LGA, using post code analysis and applying individual LGA penetration rates. Please note that some post codes expand outside the study area.

LGA	2017 players	2018 – 2028 Population growth	No. of new players in 2028
Monash	633	10,522	+34
Maroondah	304	9,995	+26
Knox	291	12,939	+23
Yarra Ranges	176	10,609	+12
Total	1,404	44,065	+95

The map on the following page provides an illustration of the projected player numbers by LGA to 2028 based on the projected population increases and current penetration rates.

Over the previous three years (since 2015) membership numbers for the four clubs located in the Outer East Region have risen from 1,155 to 1,263, an increase of 9.35% (average 3.1% per year). Using the 3.1% annual growth rate, there is likely to be an additional 379 club hockey players associated with local clubs over the next ten years.

From a facilities perspective, integrating existing provision with industry benchmarks and current and future population can provide a guide to the number of hockey pitches required in the future. The adjacent table provides pitch to population ratios for the Outer East and for each LGA.

Current pitch to population provision ratio	1 : 158,755
Maroondah City Council average pitch to population provision ratio	1: 117,396
Monash City Council average pitch to population provision ratio	1: 98,192
Knox City Council average pitch to population provision ratio	1: 163,170
Shire of Yarra Ranges average pitch to population provision ratio	0: 158,068

If no further hockey pitches are developed in the Melbourne Outer East Region before 2028, and with the loss of Knox Hockey Club, the forecast pitch to population ratio will be 1: 403,978, significantly over Hockey Victoria's recommended synthetic pitch to population benchmark of 1:100,000.

Provision ratios should be used as a guide only. Considerations such as demographics, existing facility quality and venue accessibility are not accounted for using this method.



Attachment 9.3.2

SOCIAL HOCKEY FORMATS

The way sport is being played in Australia is changing. The trend toward participation in more unstructured physical activity is evident in the 2016 AusPlay statistics, with recreational walking and gym attendances topping the list for the most popular activities for Australians.

Research undertaken by Hockey Victoria and Latrobe University in February 2018 identifies the importance of providing casual forms of hockey to meet the evolving requirements of the modern hockey participant. Two key findings from this research include:

84% of current players indicated that they would be very interested in playing at least one new format of hockey.

34% of participants highlighted the need for hockey to be offered at different times of the year.

Social Hockey is emerging as an excellent opportunity to engage both new and former hockey players to the game. Hockey Victoria, with support from VicHealth, have developed three new forms of hockey to encourage increased and new participation. These three formats are detailed in the adjacent column.

Hockey Victoria, in partnership with local clubs are investigating suitable facilities to implement social hockey programs. The preference from Hockey Victoria's perspective is to deliver these programs on an existing hockey pitch, however long pile soccer pitches or well maintained grass fields in close proximity to changerooms and toilets can also be used.

As previously mentioned, the current capacity of Melbourne Outer East hockey venues is limited, restricting the ability to support these new programs. This isn't ideal in a region with such high levels of hockey participation. For the sport to grow and develop access to existing and new facilities must be provided to enable these new social hockey formats to establish and succeed.



Aimed at past players who are not affiliated with a club and can participate without committing to regular training and competition, Hockey Sixers is a fast, fun, free-flowing version of hockey played on a smaller field, with 6 players on each team, making it more accessible and easier to get a team together.



J-Ball is delivered on a small hard court (43m x 22m) making it more geographically accessible. The game is played over 3 x 10 minute quarters with modified equipment, that is provided to each participant weekly, eliminating the high competition registration fees and need to purchase equipment.



KE40 is a new social cardio program aimed to increase and maintain fitness, and develop hockey skills. The program is delivered in conjunction with junior training and competition and is targeted at involving parents of existing participants.

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FACILITY DEVELOPMENT GUIDELINES AND RESOURCES

The Victorian Government and Hockey Victoria have developed a range of resources and sport facility guidelines that should be considered when planning for the future development of a hockey facility.

Hockey Victoria's 2014 Strategic Facilities Master Plan provides information on classification levels, facility requirements and development principles. A summary of this information is outlined below to assist Outer Melbourne East LGAs to plan for a potential new hockey facility.

Classification levels and facility requirements

State Level Facility

- Two water based synthetic pitches
- 1,000 to 2,000 lux level lighting
- Player, umpires and officials change rooms
- Covered team benches
- Media amenities
- 300 seat spectator area, electronic scoring and onsite car parking

Premier League Facility

- Water based or hybrid synthetic pitch
- Greater than 300 lux level lighting
- Player and official change rooms
- Covered team benches
- Covered or uncovered viewing areas, scoreboard, onsite car parking

Regional Level Facility

- Hybrid synthetic pitch with capacity for use by other sports
- Additional training or half pitch (desirable)
- Greater than 250 lux level lighting
- Player change rooms and team benches
- Viewing areas, scoreboard and car parking

Club Level Facility

- Hybrid or natural grass pitch (country areas)
- Greater than 200 lux level lighting
- Change rooms and team benches
- Viewing areas, scoreboard and car parking

A more detailed checklist of on-field and off-field requirements for Premier League clubs is set by Hockey Victoria under their League Entry Criteria (LEC). All clubs competing in the Premier League must have:

- A minimum of one (1) Hybrid or Water based synthetic pitch with an FIH approved surface/ product.
- A minimum of 250 lux lighting on hybrid or water based pitch.
- A minimum of two (2) fully functional change rooms per pitch, with adequate shower facilities, which are lockable, within walking distance of pitches, have hot and cold water, and electric outlets for equipment
- Covered dugouts for players and officials including seating space for minimum 5 persons and storage space for equipment. 2.
- Covered Tech Bench area with a seating space for 3 persons.

Of the four existing hockey venues in the Outer Melbourne East only one is currently occupied by a Premier League Club (Ashwood Reserve). The remaining three are all considered club level facilities.

Development principles

The following facility development principles have been identified by Hockey Victoria.

- Enhance existing hockey facility quality and capacity.
- Address the current demand and gaps in existing facility provision.
- Improve access to hockey facilities across Victoria.
- Invest in facilities that drive participation and support local competitions.
- Create flexible spaces that encourage shared and multi-purpose use and diversify program opportunities.
- Plan for new facilities in growth areas.
- Work with stakeholders to develop whole of life costs for facility development.

HOCKEY FACILITY DIMENSIONS AND SIZE REQUIREMENTS

The following information summarises the key facility elements and components of a typical one pitch hockey development.

Main pitch

- Full-size sand-dressed synthetic hockey pitch (with maximum 5m goal line and 4m sideline run-offs).
- FIH pitch accreditation.
- 1.2m concrete perimeter pathway around pitch.
- Floodlighting to a maximum level of 500 lux.
- Set of competition and training goals.
- Player / coaches boxes with drinking fountain.
- Scoreboard (electronic scoreboard is desirable).

Pavilion amenities

- 2 unisex change rooms including toilets and showers, officials change room including toilets, shower, first-aid.
- 1 social / function area.
- 1 combined canteen / kiosk and bar area.
- 1 set of male, female and accessible toilets.
- 1 meeting room / office.
- 2 internal storage areas.

Social Hockey/Junior development / training pitch (Optional) with floodlighting to maximum level of 250 lux.

Site and spectator amenities including car parking and access roads for an estimated 140 cars, outdoor covered terraced viewing area, security lighting and site and clubhouse surround landscaping.

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Indicative layout of a single pitch hockey facility with supporting amenities.

FACILITY DEVELOPMENT FRAMEWORK

Hockey Victoria's Strategic Facilities Masterplan includes a development framework and guidelines for the future planning of hockey facilities. These guidelines provide demand triggers for the establishment of new clubs through to the need for a two pitch facility, and are designed to assist clubs and local councils to plan for the future growth and development of hockey.

Investment in new, or the upgrade of existing pitch provision within the Outer Melbourne East Region should consider this development framework and aspire to achieve facility provision requirements and associated programming deliverables.

For the purposes of the Melbourne Outer East Regional Hockey Feasibility Study a training pitch is considered a fenced synthetic surface that is half or quarter of the size of a regular competition pitch. Training pitches are generally not required if two pitches can be developed on the same site.

The continued growth of Hookin2hockey and the introduction of social hockey programs provides support for the future development of multi-purpose training pitches to complement competition venues. These smaller pitches are looked on favourably by Hockey Victoria as they provide safe warm up areas and a facility for under 8's and modified hockey programs.

Although Hockey Victoria does not consider grass pitches as a suitable surface for either competition or training, grass is still relevant in some country areas and for junior programs. The preferred model is development of full size synthetic pitches that are either single or multi-purpose (depending on levels of use), and half or quarter size pitches for training or modified programs.

To develop the sport there are a number of other infrastructure elements that must be included in the design of a new hockey facility. This includes a functional pavilion located to the west of the pitch that offers a multipurpose social space, separate kitchen and bar facilities for both internal and external use, a minimum of 2 unisex change facilities to accommodate the use of one pitch, enhanced spectator amenity with covered viewing areas, accessible public toilets and car parking and school drop off zones.



SELECTING THE APPROPRIATE SITE

Site considerations and location requirements

There are a number of other factors that should be considered when selecting a site for the development of a new hockey facility. Sport and Recreation Victoria's *Artificial Grass for Sport Guide* indicates that construction costs can be severely impacted if the proposed site is not carefully selected.

Residential amenity, access, land stability and availability of services are just some of the factors that will affect the suitability of the site. The following broad criteria should be considered when planning for the future development of any new hockey facility.

- Minimum of 2.5 ha for a regional level (two pitch) facility (1.2ha for one pitch) with appropriate run offs, a junior development / training pitch, pavilion, car parking and circulation space.
- Relatively flat land with minimal trees to reduce the risk of long term problems with root damage.
- Prominent location that maximises public surveillance, exposure and awareness.
- Closely connected to public transport and/or major road networks (e.g. Eastlink).
- Connections to schools.
- Provision for potential future development (i.e. second pitch, and car park and/or pavilion expansion).
- Potential for multi-purpose pitch and/or pavilion use (if appropriate).
- Wind / element protection.
- Sealed car park and visible access.

The above information was considered in the development of more detailed selection criteria which has been used to assess potential development sites.

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Key selection criteria for the Outer Melbourne East Region

- 1. Land suitability
- Is the land suitable for the development of a hockey facility?
- · Is there available land for future expansion?
- · Are there any significant land ownership challenges?
- 2. Traffic, car parking and access
- Is the site conveniently accessible by vehicles and pedestrians?
- · Is the site located off a main road or residential street access?
- · Is there opportunity for on and off site car parking?
- 3. Usage and participation
- Will the introduction of hockey support & maximise existing use?
- If a hockey facility is closing the new site should ideally be located within 5km of the existing facility to promote hockey participant retention.
- · Is hockey considered compatible to other site users?
- 3. Local impact
- To what extent will the proximity of the proposed development and use of the site impact on local residents and neighbourhood character?
- Is there any planning or zoning issues?
- 4. Cost
- Any known site service issues likely to increase the potential cost?
- Is there opportunity to use existing amenities to reduce development costs?
- 5. Location
- Is the site considered to be in a strong catchment for participation and population growth?
- Is the site located within the accepted 20 minute drive time of the majority of current and future hockey players?
- 6. Shovel ready
- Is the site considered project ready once all approvals and funding has been secured?
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SITE ASSESSMENTS

Site options

The site selection criteria outlined on the previous page was used to conduct a preliminary desktop assessment on a range of sites nominated by Outer East Region LGAs. All sites nominated are located within the study area and are either on Council owned or managed land or education department land.

A traffic light system was used to rank each criteria as it applied to each site.

- Green Meets the key selection criteria
- Amber Partially or somewhat meets the key selection criteria
- Red Does not meet the key selection criteria

The site assessment process identified three sites as being appropriate for the future development of a hockey facility in the Outer East Region and providing a regional solution for the sport. A summary of each site including average travel distance for Knox and Croydon members is provided below.

- 1. Wantirna Reserve (Knox City Council)
- Existing sport and recreation reserve with adequate land available for potential future development of a regional level multi-sport hub.
- Good location with convenient access from Eastlink.
- Average travel distance for existing Knox members to Wantirna Reserve would be 9km.
- Average travel distance for existing Croydon members to Wantirna Reserve would be 12km.
- Site currently subject to the development of a Master Plan.

2. Ashwood Reserve - second pitch (City of Monash)

• Current hockey activity at Ashwood Reserve supports the need for a second pitch.

- Ashwood Reserve is a Premier League facility. An additional pitch would give it a more regional focus and attract users from a broader regional catchment.
- Average travel distance for existing Knox members to Ashwood Reserve would be 16km.
- Average travel distance for existing Croydon members to Ashwood Reserve would be 19km.
- It is important to note that a second pitch will not address broader regional demand or replace the future loss of the Knox pitch as the majority of Knox members live outside the 20 minute travel time.

3. Heathmont Secondary College - Maroondah (Education Department)

- School recently received \$5.5m for development of a Wellness Centre and are exploring options to develop a hockey pitch on site.
- Average travel distance for existing Knox members to Heathmont Secondary College would be 9km.
- Average travel distance for existing Croydon members to Heathmont Secondary College would be 10km.
- A master plan process will be commencing later in 2018 with hockey to be considered in the design process.
- Car parking and other sport related amenities will need to be developed which may reduce the required footprint for hockey.

The following pages provide a more detailed assessment of each site. This includes a ranking for each criteria and scaled facility development overlays in line with the dimensions and size requirements for each core facility component.

A preliminary assessment of an additional eleven (11) sites was undertaken. These are listed in the appendices section of this report.

WANTIRNA RESERVE



Mountain Highway, Wantirna
WANTIRNA RESERVE SUMMARY

The information below uses the key selection criteria to provide further detail and support for this site to accommodate the future development of a hockey facility.

Land suitability

- Wantirna Reserve is located on Crown land and is managed by Parks Victoria. Knox Council maintain and allocate use of the reserve to local sporting clubs and community groups.
- The reserve has existing sporting use and is appropriately zoned (PPRZ), providing development opportunities for additional sporting infrastructure.
- The land is large enough to accommodate one full size hockey pitch, a training pitch, on-site car parking and spectator plaza (refer Draft Concept Plans provided in the appendices section).
- The land is relatively flat with good visibility and is subject to flooding, which will need to be factored into any future design.

Traffic, parking and access

- Vehicle access is via Mountain Highway and there is ample car parking on-site that is catering for existing users.
- There is opportunity to formalise on-site parking and create in excess of 100 parking bays that will accommodate additional use.

Usage and participation

- The development of a hockey pitch on this site is compatible with existing use. Existing site users include Cricket, AFL and Tennis.
- The introduction of hockey will enhance participation at the site and complement existing use.

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The site is the closest (less than 2km) of any of the proposed sites to the existing Knox hockey facility and provides the greatest opportunity for player retention.

Local impact

- Future development of the site would not create any immediate impact on local resident. The closest residential house is approximately 400m from the site on the other side of Eastlink.
- Council is currently undertaking a Master Plan of the site which will investigate opportunities for future development and identify any local community issues.
- Cost
- There is an opportunity to share the existing sports pavilion to reduce development costs. This will require further investigation from Council

Location

- Wantirna Reserve is central to the Melbourne Outer East Region.
- The site is beside Eastlink, Dandenong Creek Trail and Eastlink Trail.

Shovel ready

- Completion of the Master Plan for the site will determine project readiness and local support for future development recommendations.
- The proposed land for the potential hockey pitch is currently underutilised as open space.

ASHWOOD RESERVE



High Street & Winbirra Parade, Ashwood



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ASHWOOD RESERVE

The information below uses the key selection criteria to provide further detail and support for this site to accommodate the future development of a hockey facility.

Land suitability

- The site is located in the City of Monash and part owned by Monash and the Crown. Local government ownership means Council would be in a position to better support the delivery of hockey.
- The club has developed plans and costings for a second pitch which is being considered by Council. The clubs concept plan for the site is provided in the appendix.
- A quarter training pitch is not be required if a second pitch was to be developed.

Traffic, parking and access

The site has vehicle access. Development of a second pitch would require a review of car parking provision and likely require additional perpendicular off street car parking along the edge of Winbirra Parade.

Usage and participation

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- The club has approximately 35 teams. Hockey Victoria have previously indicated that 30 team is considered the benchmark for consideration of a second pitch.
- There is strong demand for the development of a second pitch at Ashwood Reserve to service the current needs of the Waverley Hockey Club.
- The existing single pitch is being overused. A second pitch would address the immediate needs of the club and enable greater community use of the facility.

Waverley are the only premier league club in Melbourne's Outer East which offers a pathway for talented players. Other premier league clubs including Greensborough, Footscray and Camberwell have a two pitch facility.

Local impact

- The Victoria Sikh Association are a current tenant of the existing turf soccer pitch. Consultation with all existing user groups regarding a proposed second pitch will be required.
- Future development would require consultation with local residents.

Cost

There is an opportunity to share the existing sports pavilion to reduce development costs. The existing pavilion is ideally located between existing pitch and proposed hockey synthetic pitch to the north.

Location

Ashwood Reserve is centrally located within the Melbourne East Region and is the closest site to the CBD.

Shovel ready

The identified land for the potential hockey pitch development is being used for soccer. Further consultation and approval by Council will be required before the site is considered shovel ready.

HEATHMONT SECONDARY COLLEGE



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HEATHMONT SECONDARY COLLEGE

The information below uses the key selection criteria to provide further detail and support for this site to accommodate the future development of a hockey facility.

Land suitability

- The site is located in the City of Maroondah on education department land, limiting Councils influence over its future development and level of community use.
- The school recently received a \$5.5m funding commitment from the Victorian Government to develop a Wellness Centre on the site. The project will include an upgrade of the existing gymnasium and the introduction of a potential full size multiuse synthetic hockey pitch.
- The land identified for the potential development of a hockey facility is on the existing grass playing field. There is no room for future expansion.

Traffic, parking and access

- The school is located within a built up residential area with existing vehicle and pedestrian access.
- There is approximately 100 car parking spaces on-site. Additional car parking will be required as part of the Wellness Centre project to accommodate additional community use, potentially reducing the required footprint for hockey. CE EL CLECEL EL

Usage and participation

The current grass playing field is used by the school and for low grade community cricket over summer outside school hours.

- The introduction of a multiuse synthetic hockey pitch on the site will enable increased use by the school and offer use 12 months of the year to local clubs and community members.
- The site is less than 7km from the existing Knox hockey facility and provides opportunity for player retention.

Local impact

- The site is on education land with no planning or zoning issues.
- Future development would require consultation with local residents due to the potential impact of lights and noise as a result of a hockey facility.

Cost

There is a possibility that some of the \$5.5m commitment from the Victorian Government can be used to develop player change rooms and amenities that would be available for community use.

Location

The site is centrally located within the Melbourne Outer East Region and is minutes from Eastlink...

Shovel ready

The project will be shovel ready following final design and stakeholder approval. The school is currently working with the education department on a site plan and design of the Wellness Centre.

MANAGEMENT AND SHARED USE

There are a number of different management options that could be considered by LGAs for a proposed new hockey facility at one of the prioritised Melbourne Outer East sites. These include:

- 1. Direct management by Council
- 2. Leased directly by hockey
- 3. Shared management by hockey and another sport (via lease or license)
- 4. Representative Advisory Body under Council Management
- 5. Committee of Management
- 6. Third Party or Commercial Management (via lease).

From hockey's perspective it will provide greater benefits to the sport if the facility is leased to the tenant club all year round (Option 2). A sole lease to the hockey club will provide them with a sense of ownership and care for the facility and the ability to generate revenue throughout the year that can be invested back into the facility. Traditional hockey clubs require greater access to facilities to improve their retention and manage future growth.

If the facilities are to be shared by another user then a shared management model is recommended (Option 3). The management model would consist of medium term lease agreements between the relevant LGA, resident hockey club and a compatible and suitable co-tenant (if applicable). This would involve a shared lease between the clubs for the use of the pavilion and change rooms and separate leases for the use of the playing fields (if multiple playing fields exist). A shared lease agreement under the terms and conditions for the use of the pavilion and change rooms allows Council to transfer the general maintenance and management costs for the facility to the Clubs, and also encourages investment into the facility by the tenant Clubs.

Lease agreements will need to clearly define the broader community benefits that need to be achieved by the lessee, including the breadth of sports to be provided access to the facility, demographic or gender targets, maintenance standards and capital replacement requirements.

The individual Club lease for the use of the hockey pitch would need to consider asset renewal responsibility, which may also include a contribution to ongoing surface renewal the hockey pitch by the relevant Council.

Under a shared management model the tenant clubs would manage the facility on a daily basis and be responsible for the overall coordination and scheduling of the hockey pitch. The pavilion, change rooms and surrounding amenities, including all cleaning contracts, utility charges, asset maintenance, security, safety and risk management would also be the responsibility of the tenant clubs during their respective seasons.

As previously stated the terms and duration of these lease agreements will be determined by the relevant Council. A long term lease of 10 years or more with extension options should be considered to provide the Clubs with the security to support their long term management and financial planning and ensure stated community outcomes can be achieved.

PROVISIONAL COSTINGS

This section of the report provides recent cost estimates for the development of a purpose built hockey facility and associated amenities developed by the Knox City Council, to replace the existing Knox Hockey Club pitch at the Knox School (post October 2021). Knox City Council determined, subject to final site selection, that the establishment of a new two pitch hockey facility in Knox is estimated to cost:

Total cost estimate	\$4.5 - \$4.8m
Associated infrastructure (i.e. car parking)	\$0.5m - \$0.8m
Pavilion	\$1.5m
Lighting infrastructure	\$0.8m - \$1m
Single hybrid synthetic pitch	\$1.2m - \$1.5m

It is important to note that costs would increase if all project elements are delivered independently to potentially accommodate project staging (if desired). However, it is reasonable to assume that cost savings could be made if all, or a combination of, elements were delivered together. For example, savings could be made in site preparation works, drainage and concrete works and project management associated with both pitches being developed simultaneously.

Alternatively, should a staged approach be preferred, the installation of electrical supply and conduit for multiple pitch floodlighting may deliver an additional cost in preliminary stages, but savings will be generated in the long-term with preparatory cabling already laid ready for connection at a later date. Costs may also be reduced if there is existing infrastructure on the site, such as a pavilion or car park, that could potential be co-shared with the hockey club. A budget for the upgrade or expansion of an existing pavilion would most likely need to factored in to accommodate the needs of hockey.

Lifecycle costs

Facility development planning must also consider ongoing maintenance expenses, as well as life-cycle and replacement costs of all hockey pitch elements, including pitch surfaces, floodlighting, fencing and equipment. In most instances, club based hockey facilities (under volunteer management), do not make sufficient revenue or retain a financial surplus to support the full cost of maintaining and replacing synthetic surfaces. As a result, the majority of clubs require the ongoing support of their land owner (generally Council), to continually provide quality infrastructure for community use.

The expected life of a hockey pitch, depending on level of usage, is between 10 and 12 years. It is expected that within a 30 year period a hockey pitch surface will need to be replaced at least twice. Replacement of pitch elements vary, from a number of years for highly used equipment such as goals, from 10 to 12 years at the time of surface replacement, through to 30 years for lighting poles and fixtures.

It will be important for LGAs to consider these future costs when structuring management and operating agreements and determining the usage fees and charges for the club and alternative hirers. The following information provides current high level life-cycle costs for key hockey pitch infrastructure and related elements over a 10 year period. These cost are in addition to the annual maintenance budget required for the facility.

Total replacement cost (after 10 years)	\$685,000
Goals	\$25,000
Fence replacement	\$50,000
Shock pad	\$75,000
Uplift and disposal of surface	\$35,000
Synthetic pitch surface replacement	\$500,0000

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FUNDING OPPORTUNITIES

Maximising facility use and the ability to demonstrate increased participation opportunities and physical activity benefits through the development of a hockey pitch will be critical in being able to attract state government funding for the project.

Sport and Recreation Victoria (SRV) provide funding options that should be considered to support the development of a funding model for a hockey project. These include the Community Sports Infrastructure Fund Program and the Community Sports Infrastructure Loans Scheme.

The Community Sports Infrastructure Funding Program (Major Facilities) provides grants of up to \$800,000 for construction of sub-regional and regional level sports facilities. To strengthen the identified outcomes from this funding program, Councils are required to engage SRV and other key stakeholders as early as possible to develop proposals that strongly align with the fund. To be eligible for this grant the applicant must provide:

- Site specific plan / aerial map showing location of proposed project.
- Schematic Plans (site specific) developed with SRV and stakeholder input.
- Detailed area schedule for prefabricated/modular construction projects.
- Quantity survey, tender price or independent qualified expert report are required for projects with a total project cost over \$1 million.
- Evidence of confirmation of funding sources (e.g. council report confirming contribution, letter from council CEO or club bank statements).
- Environmentally Sustainable Design report(s) and budget.
- Project Management Framework.

Under this funding program SRV will provide \$1 for every \$3 contributed to the project from the local Council or other external funding providers. Other potential funding providers to the development of a new hockey facility to replace the planned closure of the Knox Hockey Pitch would be Knox City Council (via their capital works program) and the Knox Hockey Club who have access to a \$400,000 sinking fund.

It is recommended that a condition of any hockey club lease includes a requirement of the Club to contribute to a sinking fund for future facility replacement. To ensure the Club can meet these requirements and ongoing maintenance responsibilities a whole of facility financial model should be considered as part of the Clubs lease agreement.

SRVs Community Sports Infrastructure Loans Scheme provides organisations access to low interest rate loans between \$500,000 and \$10 million to deliver community sport and recreation infrastructure.

The Loans Scheme can fund 100 per cent of the total project cost and be secured in conjunction with a government grant, ensuring that any grant funding program ratios are still met with the required cash contributions.

Priority will be given to the projects that provide the greatest community benefit. Of particular relevance to hockey is that the Loans Scheme can be used for projects involving synthetic sports surfaces (including replacements) and new or upgraded community sport precincts, including sports fields and lighting. Applications will consist of three components. All components must be provided in order for an application to be deemed eligible for assessment. Applicants are required to complete the loan application form and provide a supporting business case. External funding opportunities for hockey projects will be enhanced if the project can demonstrate sharing of facilities with other sports and activities. Maximising the use of the space and encouraging greater participation opportunities for females and junior or modified programs are generally looked on favorably.

In regards to sharing a synthetic hockey pitch, lacrosse is considered a compatible sport due to its usage requirements. If demand in the area grows then the use of a new hockey facility may become a viable option for lacrosse. If this occurs, minor modifications to the pitch dimensions should be considered by the relevant LGA prior to development. Modified versions of lacrosse (e.g. Box Lacrosse) can also be played in smaller areas, similar to that of the hockey player development/training pitch.

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Attachment 9.3.2



SUMMARY OF PRIORITIES FOR HOCKEY IN THE OUTER EAST



PRIORITY RECOMMENDATIONS: YEAR 1

The following strategic actions have been identified for the region and are considered critical to the successful planning, funding and delivery of hockey related projects within the Melbourne Outer East Region. It is recommended that all stakeholders in the Region acknowledge recommended actions and seek collaborative approach to their future delivery.

NO	ACTIONS	TIMEEDAME	STAKEHOLDER RESPONSIBLE		
NU.	ACTIONS		INITIATE	DELIVER	SUPPORT
1.	Conduct further investigation, consultation and detailed planning for the development of a hockey venue to replace the Knox Hockey Club pitch at Knox School at one of the following preferred site options – Wantirna Reserve or Heathmont Secondary College. **	Year 1 (Immediate term)	Knox Council, Maroondah Council, Heathmont Secondary College	Knox Council, Maroondah Council, Heathmont Secondary College	Knox Hockey Club, Hockey Victoria
2.	Following agreement on the most suitable site for the Knox Hockey Club, develop a three-year relocation plan that includes a design and development program, funding model, management and operational plan, consultation and engagement process and costed design drawings.	Year 1 (Immediate term)	Knox Council, Maroondah Council, Hockey Victoria	Knox Council, Maroondah Council	Knox Hockey Club
3.	Establish stronger partnerships with the education sector and provide in principle support for the proposed future development of multi-use synthetic pitches, suitable for hockey competition at Heathmont Secondary College.	Year 1 (Immediate term)	Maroondah Council, Hockey Victoria	Maroondah Council	Department of Education and Training
4.	Undertake a feasibility study with the view to developing a sustainable funding and management model (capital and operational) for a second competition level synthetic pitch at Ashwood Reserve.		Monash Council, Hockey Victoria, Waverley Hockey Club	Sport and Recreation Victoria, Monash Council	Hockey Victoria, Waverley Hockey Club
5.	Partner with the education sector to secure a longer-term occupancy arrangement for the Croydon Hockey Club at Yarra Valley Grammar School through the provision of upgraded player and spectator amenities.	Years 2 - 4 (Medium term)	Maroondah Council, Croydon Hockey Club	Maroondah Council, Yarra Valley Grammar School	Department of Education and Training, Hockey Victoria and Croydon Hockey Club

** A new hockey pitch would ideally be developed on Council owned / managed land within the City of Knox and in a location easily accessible to existing Knox Hockey Club members.

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PRIORITY RECOMMENDATIONS: YEARS 2 - 4

NO	ACTIONS	TIMEEDAME	STAKEHOLDER RESPONSIBLE		
NO.	ACTIONS		INITIATE	DELIVER	SUPPORT
6.	Undertake a localised demand analysis study to determine the trend in Hockey participation rates in Yarra Ranges.	Years 2 - 4 (Medium term)	Hockey Victoria, Yarra Ranges Council	Sport and Recreation Victoria, Yarra Ranges Council	Hockey Victoria
7.	Secure a sustainable funding model (capital and operational) and deliver a new hockey venue to replace the loss of the Knox Hockey pitch at Knox School, which is scheduled to close in October 2021.	Years 2 - 3 (Medium term)	Knox Council, Hockey Victoria	Sport and Recreation Victoria, Knox Council, Maroondah Council	Hockey Victoria, Knox Hockey Club
8.	Subject to findings / recommendations of the Monash City Council Ashwood Reserve Feasibility Study, undertake detailed design for a second competition level synthetic pitch with floodlighting at Ashwood Reserve.	Years 2 - 4 (Medium term)	Monash Council, Hockey Victoria, Waverly Hockey Club	Sport and Recreation Victoria, Monash Council	Hockey Victoria, Waverly Hockey Club
9.	Engage with clubs to review existing occupancy arrangements and pitch schedules to determine available times to introduce social hockey formats to the region. Support Hockey Victoria marketing strategies to promote these new formats to existing non-hockey participants.	Years 2 - 4 (Medium term)	Hockey Victoria	Hockey Victoria	All Outer East Councils and Clubs
10.	Provide support, development and occupancy advice to the Heathmont Secondary College on the proposed development of their multi-use pitch. Floodlighting, player and spectator amenities and car parking must be provided for this pitch to be suitable for club use.	Years 2 - 4 (Medium term)	Hockey Victoria, Maroondah Council	Maroondah Council, Heathmont Secondary College	Hockey Victoria, Department of Education and Training

PRIORITY RECOMMENDATIONS: YEARS 5 - 10

NO	ACTIONS		STAKEHOLDER RESPONSIBLE		
NO.		TIMEFRAME	INITIATE	DELIVER	SUPPORT
11.	Subject to the findings / recommendations of the Monash City Council Ashwood Reserve Feasibility Study and provision of funding, construct a second competition level synthetic pitch with floodlighting at Ashwood Reserve.		Monash Council, Hockey Victoria and Waverly Hockey Club	Sport and Recreation Victoria, Monash Council	Hockey Victoria, Waverly Hockey Club
12.	Review hockey trends and re-audit existing facilities to obtain current participation, facility and demand information to inform future decision making.	Years 5 - 10 (Long term)	Hockey Victoria	Hockey Victoria	All Outer Melbourne East Councils



OTHER SITE ASSESSMENTS

The following nominated sites located within the study area were assessed against the agreed criteria to determine their suitability for the future development of a hockey facility. A copy of these site assessment can be provided following approval from the PCG.

Knox

- Knox Regional Sports Park
- JW Manson Reserve

Maroondah

- Quambee Reserve Horse & Pony Club
- Melba Secondary College
- Dorset Recreation Reserve
- Eastfield Park
- Greenwood Avenue

Yarra Ranges

- Yarra Hills Secondary College
- Kilsyth Recreation Reserve

Monash

- Drummies Bridge Reserve
- Gladeswood Reserve







ASHWOOD RESERVE SCHEDULE – PITCH 1

ekly	Calendar - Pitch 1			1		1	
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-30							
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:45 am						Doncaster	
:15						U10 Boys & Girls v	
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:15						Hard States and	
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pm						Fanulers	
30						-	100 C 100
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30 35			Mens PD, PE, PF, MetA				
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ASHWOOD RESERVE SCHEDULE – PITCH 2

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ABOUT THIS DOCUMENT

This study was commissioned by four of the seven Melbourne East Local Government Authorities (Maroondah City Council, Knox City Council, Yarra Ranges Council and City of Monash) after identifying a need to plan for the future provision of hockey across the Region.

Input and advice from the Project Control Group, consisting of representatives from the above organisations has overseen the development and delivery of this document, with support from Hockey Victoria and local clubs.







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Attachment 3



KNOX HOCKEY FACILITY FEASIBILITY STUDY

FEASIBILITY STUDY

Issue 001 | Revision B | 16/12/2019 | Ref: 10498-00

Author: Jess Sewell

This report takes into account the particular instructions and requirements of our client. It is not intended for and should not be relied upon by any third party and no responsibility is undertaken to any third party.



16/12/2019

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Appendices

Appendix A - Site Photos Appendix B - Detailed Feature Surveys Appendix C - Geotechnical Investigations Appendix D - Environmental Site Assessment Appendix E - DBYD Asset Plans Appendix F - Development Sketches Appendix G - Cost Estimates



1

Knox Hockey Facility Feasibility Study

16/12/2019

1. Introduction

1.1 General

SPORTENG have been commissioned by Knox City Council (Council) to undertake a feasibility study and concept design for the development of the Knox Hockey Facility, within the Knox City Council municipality. The study investigates the feasibility of the development of the facility at the following sites:

- Benedikt Reserve 39-53 Rosehill St Scoresby, Victoria 3179
- Wantirna Reserve located off Mountain Highway Wantirna, Victoria 3152

The intention of the assessment is to undertake a feasibility study and produce concept designs for the following:

- One synthetic grass hockey field
- New fencing and lighting
- Site access including carparking and D.D.A compliant access to the facility
- Pavilion

This report will cover the following:

- Overview of the existing site
- Site investigation and key findings
- Development options



1.2 Existing Sites

Two locations within the municipality have been identified as potential development sites:

Benedikt Reserve (39-53 Rosehill St Scoresby)



Figure 1 - Aerial photograph of Benedikt Reserve

The facility consists of the following:

- Synthetic cricket wicket
- AFL goal/ point posts
- Multi-use acrylic court
- Playground
- Pavilion and amenities block
- Formalised carpark
- Footpath network

Wantirna Reserve (Mountain Highway Wantirna)

- The facility consists of the following:
 - Undeveloped green field
 - Formalised AFL/ cricket oval
- Cricket nets
- Tennis Club
- Scout Hall
- Playground
- Unsealed carparks
- Connectivity to Dandenong Creek Trail



Figure 2 - Aerial photograph of Wantirna Reserve

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2. Feasibility Overview

2.1 General

The site investigation consisted of the following for each respective site:

- Visual assessment
- Detailed feature survey
- Geotechnical investigation
- VicPlan assessment
- High level assessment of existing in-ground services

2.2 Visual Assessment

SPORTENG undertook a site inspection of the two sites, Benedikt Reserve and Wantirna Reserve. The assessment consisted of a visual inspection of the site to identify any site constraints that may impact on the proposed development of the site.

Refer to Appendix A for site photos.

2.3 Detailed Feature Survey

The detailed feature survey was undertaken by Surfcoast Survey & Drafting Service for both sites and consisted of the following:

• Detailed site survey to establish existing conditions

Refer to Appendix B for detailed feature surveys.

2.4 Geotechnical Investigation

Benedikt Reserve:

Ground Science conducted a geotechnical investigation of the Benedikt Reserve site on the 30th October 2019. The investigation consisted of the following:

- Drilling of 8 boreholes to a target depth of 1.5m to 2.5m below the surface level and collection of soil samples
- Laboratory testing of samples
- Preparation of report detailing geotechnical investigation, findings and recommendations for the site



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Wantirna Reserve:

Council provided the following geotechnical investigation and environmental site assessment reports for review:

- Proposed Eastern Football Hub, Wantirna South Recreation Reserve Geotechnical Investigation (Preliminary), 115706. Dated the 24th March 2014 by A.S. James PTY Limited
- Preliminary Environmental Site Assessment, Wantirna Reserve Former Landfill. Dated 16 August 2019 by AECOM Services Pty Limited

Refer to Appendix C for geotechnical investigation reports and Appendix D for environmental site assessment report.

2.5 VicPlan Assessment

SPORTENG conducted a desktop assessment using Victorian State Government Environment, Land, Water and Planning electronic asset VicPlan to identify relevant land management overlays such as Land subject to inundation overlay (LSIO).

2.6 High Level Assessment of Existing In-Ground Services

Review of Dial Before You Dig (DBYD) documentation to identify any major authority underground assets.

Refer to Appendix E for relevant DBYD PDF asset plans. Note, indicative asset location only. Utilities to be traced and/ or potholed to determine impact on proposed works.



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3. Assessment

3.1 Benedikt Reserve

3.1.1 Visual Assessment

Based on the visual assessment of the Benedikt Reserve site, the following key observations were noted:

- Located within a sports/ recreation hub;
 - Natural turf open field (including synthetic cricket wicket)
 - o Multiuse court; tennis and half -court basketball
 - o Playground
 - o Pavilion and amenities block
- No sign of formal irrigation or drainage for turf open field
- Existing lighting infrastructure
- Existing AFL goal/ point posts within the field of play
- Existing path network throughout entirety of site
- Proximity/ visibility from surrounding residential road network
- No frontage to main road network, set back from Stud Road and Ferntree Gully Road within existing residential suburb of Scoresby
- Limited on-street parking
- Opportunity for off-street parking at south-east of site
- Mature trees

3.1.2 Detailed Feature Survey

Based on review of the detailed feature survey, the following was noted:

- Existing grades of the development area do not exceed governing body requirements
- Trees are located outside the development area
- Adequate spatial requirement for development of hockey field
- Survey indicates the site is serviced by the following assets:
 - Electrical low voltage electrical conduits servicing existing light towers in vicinity of field of play
 - o Gas
 - o Sewer
 - o Stormwater drainage
 - o Potable water

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3.1.3 Geotechnical Investigation

The following key findings were noted for the geotechnical investigation:

- Relatively consistent soil profile present, comprising 150mm 300mm of organic topsoil over a silty clay material, "Humevale Formation" deposits
- The results of the laboratory testing and field tests indicate that the natural soils are considered highly reactive and likely to be subject to shrinking and/ or swelling movements
- Construction timing identified as a geotechnical risk, due to impacts of weather on the highly reactive subgrade. Construction recommended during dry, summer months

Pavement design as per Ground Science's recommendations.

3.1.4 VicPlan Assessment

Based on the desktop assessment of the VicPlan asset map the Benedikt Reserve site is outside of any land management overlays, including:

- Floodway overlay
- LSIO

3.1.5 High Level Assessment of Existing In-Ground Services

The following key observations were noted from the assessment of Dial Before You Dig PDF asset documents:

- Assets with existing site connections;
 - o Electrical
 - o Stormwater drainage
 - o Potable water
- Assets present in surrounding areas;
 - o Communications, including NBN
 - o Gas
 - o Sewer
- Asset mains do not appear to cut through the development area



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3.2 Wantirna Reserve

3.2.1 Visual Assessment

Based on the visual assessment of the site, the following key observations were noted:

- Located within a sports/ recreation hub;
 - Adjacent natural turf oval with synthetic cricket wicket and synthetic cricket nets adjacent to development area
 - o Pavilion
 - o Tennis courts, Wantirna Tennis Club
 - o Playground
 - Scout Hall
- Proposed development area an existing green field site no disruption to existing formal user groups
- Proximity to adjacent high voltage electrical easement
- Existing unsealed carparks and access roads
- Signs of pavement movement observed in sealed access road and round about
- Natural swale drains with minimal formalised drainage infrastructure observed on site
- Connectivity with Dandenong Creek Trail
- Offset from Mountain Highway, no direct visibility from road network requires signage
- Well connected with the arterial road network i.e. access from Eastlink, Mountain Highway and Burwood Highway

3.2.2 Detailed Feature Survey

Based on review of the detailed feature survey, the following was noted:

- Existing grades of the proposed development area do not exceed governing body requirements
- Trees (11) located within the proposed development area
- Adequate spatial requirement for development of hockey field
- Survey indicates the site is serviced by the following assets:
 - o Stormwater drainage



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3.2.3 Geotechnical Investigation

The following key findings were noted for the geotechnical investigation:

- Site was a former landfill with the exact size, extent and depth of fill not known
- The location of the proposed hockey pitch would be located over the area identified in the environmental report as the 'Northeast reworked landfill' and was operation between 1968 and 1975
- The site ownership is crownland
- Anecdotal evidence suggests that the former landfill accepted municipal waste however other waste may have been accepted
- Gas monitoring of existing structures and pits did not observe methane readings significantly greater than typical ambient readings
- Due to the presence of the underlying fill, there is a high possibility that the ground will settle over time this will ultimately impact the performance of the overlying pavement if not properly addressed

Pavement design - Two (2) options to consider;

- Option 1 (high risk of pavement settlement)
 - Geo-composite membrane
 - 2 x 200mm capping layer with tensar grid between both layers
 - o 200mm class 2 crushed rock
- Option 2 (low risk of pavement settlement)
 - o Rigid pavement with piles

3.2.4 VicPlan Assessment

Based on the desktop assessment of the VicPlan asset map the Wantirna Reserve site is within the following land management overlays:



- LSIO light blue
- Borders an environmental significance overlay area (ESO2) lime green
- AAV Cultural Heritage area (200m offset from waterway) – a Cultural Heritage Management Plan (CHMP) required for proposed development
- Bushfire prone area

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Knox Hockey Facility Feasibility Study

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Figure 3 - VicPlan map of Wantirna Reserve with LSIO and ESO2 overlays

3.2.5 High Level Assessment of Existing In-Ground Services

The following key observations were noted from the assessment of Dial Before You Dig PDF asset documents:

- Assets with existing site connections;
 - o Stormwater drainage
- Assets present in surrounding areas;
 - Communications, including NBN Connections to adjacent oval and tennis centre pavilions
 - Electrical Connections to adjacent oval and tennis centre pavilions
 - o Potable water Connections to adjacent sites
 - o Sewer Connections to adjacent sites
- Asset mains do not appear to cut through the development area



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4. Findings

4.1.1 Benedikt Reserve

Based on the assessment of the Benedikt Reserve site the key findings are summarised in the table below.

Table 1 - Benedikt Reserve site advantages and disadvantages

Advantages	Disadvantages
 Located within an existing sports hub with adjacent oval and tennis court Existing pavilion building – suggests sufficient services to site Relatively flat terrain Existing natural turf playing space has lighting – residents accustomed to sport field lighting Playground adjacent to site Not within a flood overlay area Good level of passive surveillance No tree removal required Typical construction program - standard construction techniques, pavement tolerances maintained 	 Located within a residential area; noise and light overspill may be of concern May impact on the current sporting groups / users of the facility (i.e. dog walkers) Limited space for off-street parking Neighbourhood road access only

4.1.2 Wantirna Reserve

Based on the assessment of the Wantirna Reserve site the key findings are summarised in the table below.

Table 2 - Wantirna Reserve site advantages and disadvantages

Advantages	Disadvantages
 Located within an existing sports hub consisting of adjacent oval, cricket nets and tennis courts Adequate space for off-street parking Undeveloped land Relatively flat terrain Will not impact existing sporting groups / users 	 Built over an old landfill – signs of subsidence in existing sealed and unsealed pavements Proximity to high-voltage overhead wiring Requires removal of trees and vegetation Potentially limited existing in-ground services for proposed pavilion Limited existing infrastructure Located within a flood overlay area Limited passive surveillance Limited opportunity to expand Requires planning permit



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5. Development Options

5.1 **Project Requirement**

Based on the site constraints observed, including tree retention value and spatial availability, there is limited layout options for the proposed works. SPORTENG have developed layouts for each site with the following aims:

- Provide a 'Minimum' FIH standard multi-use synthetic grass hockey field with provision for;
 - o Hockey half field use
 - o Soccer
 - o Lacrosse
- Provide the following associated facility infrastructure, including;
 - Pavilion, size based on Knox City Council Guidelines for Developing Sports Facilities room schedule
 - o Carparking
 - o D.D.A compliant access between carparking, pavilion and hockey field

Refer to Appendix F for proposed development sketches.

The construction of the hockey pitch pavement and pavilion for Benedikt Reserve is not considered complex consisting of a conventional flexible pavement with capping layer. Ground conditions at Wantirna Reserve have, however dictated two potential solutions:

- Flexible pavement consisting of deep bridging layer high risk and therefore not considered further
- Piled rigid pavement low risk and the recommended option to be considered for the Wantirna Reserve site

Scope of works involved are considered in more detail in the following section.


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5.2 Scope of Works

5.2.1 Benedikt Reserve

The scope of works for the Benedikt Reserve site and associated costs are tabulated below regarding:

- Field of play scope of works, and
- Pavilion extension scope of works

Refer to Appendix G for a detailed breakdown and assumptions and qualifications.

Table 3 - Benedikt Reserve field of play scope of works and associated costs

Field of Play Scope of Works	Cost (excl. GST)
Construction Preliminaries	\$130,530
Demolition: • Remove and dispose off-site redundant in-ground services • Break out and dispose off-site existing concrete pavement • Remove and dispose off-site existing AFL goal/ point posts • Break out and dispose off-site existing light towers • Break out and dispose off-site existing synthetic cricket wicket Excavation: • Strip organic layer and dispose off-site • Cut to subgrade levels and dispose off-site • Prepare subgrade (final trim)	\$135,980
 In-ground Services: Drainage: Install hockey field drainage network and connect to existing LPoD Electrical: Install L.E.D lighting and posts (including footings) 	\$587,950
 Pavements & associated elements: Field of play works; Construct synthetic pitch Place and compact crushed rock layer (including capping layer) Install FIH approved multi-use synthetic surface system including line marking Reinstate natural turf Construct hard pavements, including Asphalt carpark and access road Concrete pedestrian paths and hard stand Concrete works; Perimeter concrete edge strips/ kerb and channel 	\$1,252,560
Miscellaneous: • Fencing: • Install perimeter black chain mesh fencing (various heights) • Install pedestrian and vehicle gates with grated shoe cleaners • Sports Equipment: • Install hockey goals (including associated footings) removable • Install players and officials shelters	\$193,330
Subtotal (excl. GST)	\$2,300,350
Contingency (20%)	\$460,070
Design consultancy (7%)	\$161,025
Total	\$2,921,445



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Table 4 - Benedikt Reserve pavilion scope of works and associated costs

Pavilion Extension Scope of Works	Cost (excl. GST)
Construction Preliminaries	\$76,440
Demolition: • Breakout bench seat and return to Council Excavation: • Strip organic layer and dispose off-site • Prepare subgrade (final trim)	\$2,800
In-ground Services: • Pavilion Supply; • Electrical Services • Communications • Sewer • Hydraulics	\$20,000
Pavilion: • Pavilion extension	\$1,250,000
Subtotal (excl. GST)	\$1,349,240
Contingency (20%)	\$269,848
Design consultancy (7%)	\$94,447
Total	\$1,713,535

Total

Benedikt Reserve pavilion extension is based on Knox City Council Guidelines for Developing Sports Facilities room schedule and the following methodology:

- Existing two change rooms and amenities to be retained and configured to allow each room to be divided into two and allow separate male and female change area and toilets/ showers
- Existing amenities to be extended (convert existing kitchen and storage rooms)
- Extension to provide for;
 - o Medical/ trainer's room
 - o Kitchen and servery
 - o Social space
 - o Office
 - o Storage rooms, including cleaners store
- Existing toilet block to be retained .

5.2.2 Wantirna Reserve

The scope of works for the Wantirna Reserve site and associated costs are tabulated below, regarding:

- Field of play scope of works, and
- Pavilion scope of works

Refer to Appendix G for a detailed breakdown and assumptions and qualifications.

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 Table 5 - Wantirna Reserve field of play scope of works and associated costs

Field of Play Scope of Works	Cost (excl. GST)
Construction Preliminaries	\$210,030
Demolition: • Cut down and dispose of trees (including root system) • Break out existing unsealed carpark and dispose off-site <u>Excavation:</u> • Strip organic layer and dispose off-site • Cut to subgrade levels and dispose off-site • Prepare subgrade (final trim)	\$129,250
 In-ground Services: Drainage; Install hockey field drainage network and connect to existing stormwater drainage infrastructure Electrical; Site supply connection Install L.E.D sports lighting (including footings) 	\$682,450
 Pavements & associated elements: Field of play works; Construct synthetic pitch; Rigid pavement with piles (incl. topping slab) Install FIH approved multi-use synthetic surface system including line marking FIH compliance testing Construct hard pavements, including; Concrete pedestrian paths and hard stand Concrete works; Perimeter concrete edge strips/ kerb and channel 	\$2,504,740
Miscellaneous: • Fencing: • Install perimeter black chain mesh fencing (various heights) • Install 1.2m wide pedestrian gates with grated shoe cleaners • Install 4.0m wide vehicle gate with grated wheel cleaner • Sports Equipment: • Install hockey goals (including associated footings) removable • Install players and officials shelters • Other: • Root barrier	\$190,000
Subtotal (excl. GST)	\$3,716,470
Contingency (20%)	\$743,294
Design consultancy (7%)	\$260,153
Total	\$4,719,917



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 Table 6 - Wantirna Reserve pavilion scope of works and associated costs

Pavilion Scope of Works	Cost (excl. GST)
Construction Preliminaries	\$214,150
Excavation: • Strip organic layer and dispose off-site • Cut to subgrade levels and dispose off-site • Prepare subgrade (final trim)	\$2,100
In-ground Services: • Pavilion Supply; • Electrical Services • Communications • Sewer • Hydraulics	65,600
Pavilion: • Construct new pavilion – piled	\$3,500,000
Subtotal (excl. GST)	\$3,781,850
Contingency (20%)	\$756,370
Design consultancy (7%)	\$264,730
Total	\$4,802,950



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6. Conclusion

Both proposed sites have adequate space for the proposed development of a FIH complaint hockey pitch facility. There are key limitations for both sites:

- Benedikt Reserve:
 - o Loss of open natural turf recreational space
 - o Situated within a residential area
- Wantirna Reserve:
 - o Former landfill site
 - o Located within a flood water inundation overlay

The following table summaries the cost estimate for each site (including field of play and pavilion elements).

Table 7 - Comparison of Construction Cost Estimates

Scope of Works	Field of Play		Pav	vilion
	Benedikt Reserve	Wantirna Reserve	Benedikt Reserve	Wantirna Reserve
Construction Preliminaries	\$130,530	\$210,030	\$76,440	\$214,150
Demolition & Excavation	\$135,980	\$129,250	\$2,800	\$2,100
In-ground Services	\$587,950	\$682,450	\$20,000	\$65,600
Pavements & associated elements	\$1,252,560	2,504,740	-	-
Miscellaneous	\$193,330	190,000	-	-
Pavilion	-	-	\$1,250,000	\$3,500,000
Subtotal (excl. GST)	\$2,300,350	\$3,716,470	\$1,349,240	\$3,781,850
Contingency (20%)	\$460,070	\$743,294	\$269,848	\$756,370
Design consultancy (7%)	\$161,025	\$260,153	\$94,447	\$264,730
Total	\$2,921,445	\$4,719,917	\$1,713,535	\$4,802,950

Redevelopment of Benedikt Reserve appears to be the most cost-efficient solution. Environmental factors, including existing ground conditions and site overlays indicates Wantirna Reserve is a higher risk, higher a cost option.



16/12/2019

Appendix A - Site Photos



16/12/2019

Benedikt Reserve, Scoresby

The following photographs provide a high-level overview of the Benedikt Reserve site.



Figure 1 - Existing changeroom / pavilion



Figure 2 - Existing natural turf playing field





Figure 3 - Flat parcel of land adjacent fields could be Figure 4 - Multi-use courts adjacent to proposedused for off-street parkinghockey pitch location



Figure 5 - Existing public amenities block



Figure 6 - Multi-use courts adjacent to proposed hockey pitch location



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Wantirna Reserve, Wantirna

The following photographs provide a high-level overview of the Wantirna Reserve site.



Figure 7 - Existing unsealed carpark



Figure 8 - Undeveloped land – potential site for hockey pitch



Figure 9 - Existing cricket practice nets adjacent to Figure 10 - Adjacent high-voltage overhead wires proposed hockey pitch location



adjacent to site



Figure 11 - Existing unsealed carpark and pavilion servicing the natural turf oval



Figure 12 - Undeveloped land – potential site for hockey pitch



16/12/2019

Appendix B - Detailed Feature Surveys







16/12/2019

Appendix C - Geotechnical Investigations





GEOTECHNICAL INVESTIGATION BENEDIKT RESERVE, SCORESBY

Prepared for SPORTENG Pty Ltd

Report Reference: G4091.1 AA

Date: 2 December 2019

ABN 31 105 704 078 13 Brock Street, Thomastown Victoria 3074 (P) +61 3 9464 4617 (F) +61 3 9464 4618



PROJECT DETAILS

Project Reference	G4091.1	Rev	AA
Project Title	Benedikt Reserve		
Project Location	39-53 Rosehill Street, Scoresby	State	VIC
Date	2 December 2019		

CLIENT DETAILS

Prepared For	SPORTENG Pty Ltd		
Client Address	Level 1, 206 Camberwell Road	Suburb	Hawthorn East

DISTRIBUTION

Original Held By	Ground Science Pty Ltd
One (1) Electronic Copy	SPORTENG Pty Ltd

This document presents the results of the geotechnical investigation conducted for the aforementioned project and is detailed for the sole use of the intended recipient. Should you have any questions related to this report please do not hesitate to contact the undersigned.

Author:

Gee Singh Senior Geotechnical Engineer

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1. INTRODUCTION

Ground Science have prepared this report to present the results of a geotechnical site investigation carried out for the project identified as Benedikt Reserve, located at 39 – 53 Rosehill Street in Scoresby, Victoria (the site). The scope of works detailed herein was commissioned by SPORTENG Pty Ltd (the Client) through acceptance of our fee proposal, GSP2019355 AA dated 22nd October 2019.

2. PROJECT BACKGROUND & UNDERSTANDING

We understand that the project involves the development of a synthetic playing field, which will involve conversion of the northern portion of the existing natural turf field into a synthetic hockey playing field. An overview of the site and the proposed development footprint is shown in Figure 1 below:



Figure 1: Overview of Site & Approximate Development Footprint

Ground Science were commissioned to undertake a geotechnical investigation to assist the civil and structural development of the above site, including provision for preliminary design recommendations for the proposed synthetic pavement profile.

The professional advice provided in this report is based on the information provided at the time of the report preparation and may not be valid if changes are made to the site, the development proposal or the construction methods. In the event of such changes, further advice should be sought from Ground Science.

3. SCOPE OF WORKS

The scope of works for the project generally involved the following:

- Drilling of 8 boreholes using solid flight auguring techniques to target depths of between 1.5m and 2.5m;
- Dynamic Cone Penetrometer (DCP) tests performed adjacent to each test location;
- · Recover of soil samples from selected test locations and/or soil horizons for laboratory testing;
- Documentation of site features, general observations and photographs during the fieldwork.



The boreholes were advanced using a 4WD mounted ATS Drilling Rig supplied and operated by Ground Science. A suitably qualified geotechnical engineer undertook the fieldwork, located the boreholes, performed insitu testing, logged the soils encountered and recovered soil samples. The laboratory tests were performed in Ground Science's NATA accredited testing facility in Thomastown.

4. RESULTS

4.1 REGIONAL GEOLOGICAL & GROUNDWATER CONDITIONS

Available geological information suggests the site is underlain by Devonian aged 'Humevale Formation' deposits, typically characterised as Sandstone/Siltstone overlain by associated residual soils. Available aerial imagery (obtained from Nearmap) indicates the site has remained relatively unchanged over the last 10 years, with the exception of a multi-sport paying surface constructed to the north east, however this is noted to be outside of our investigation area.

The Visualising Victoria's Groundwater (VVG) online database indicates the regional groundwater table to be located at depths exceeding 10m below surface levels. Information from nearby bores within the site or surrounding areas was not available for review.

We also understand that a low voltage (LV) service asset transects the site and the boreholes were positioned outside of such areas under the direction of an accredited utility asset locator. The approximate location of the LV service is shown on Figure 2 in Appendix A. Other service assets are also known to exist (not shown on Figure 2). It should therefore be noted that some variability in the soil profile described in the following sections may be present within trench backfill materials.

4.2 SURFACE CONDITIONS

Benedikt Reserve is situated within a residential zone and is bordered by Benedikt Court to the north, existing reserve/hard courts to the east, residential properties to the south and west. The site currently comprises of a turf field and associated club rooms/buildings on the east. Large rows of trees line the perimeter of the existing field along the east (extensively) and sporadically on the south, west and northern perimeters.

Topographically, the site was observed to be relatively flat. Minor embankments/slopes are noted along the perimeters outside of the investigation zone, generally within the existing hard-court playing area on the east. At the time of our investigation, the site was trafficable to a 4WD drilling rig. Site photographs taken during our investigation are presented in Appendix D of this report.

4.3 SUBSURFACE CONDITIONS

The subsurface soil profile encountered in the boreholes is shown on the engineering borehole logs in Appendix B of this report. A summary of the subsurface soil profile is provided below:

<u>Unit 1: Topsoil</u>

Unit 1 topsoil was encountered at all test locations and generally recovered as Silty CLAY (CL-CI), low plasticity, pale to dark brown, with rootlets/organics, trace sand, firm to stiff and dry.

Unit 2: Residual 'Humevale Formation' Deposits

Unit 2 Residual 'Humevale Formation' deposits were encountered at all test locations and generally recovered as Silty CLAY (CI-CH), medium to high plasticity, brown mottled grey, with trace rootlets (in the upper soil horizons), with gravel inclusions. The Unit 2 soils were typically firm to very stiff and dry to damp.

All boreholes were terminated within the Unit 2 deposits, with all boreholes achieving the target depths. Groundwater was not encountered in the boreholes during our investigation. It should be noted that groundwater can vary seasonally and with changes to drainage conditions.



A summary of the depth range of each soil unit, is shown in Table 1:

I a satism	Unit 1: Topsoil	Unit 2: 'Humevale Formation'	Completion
Location	Depth Range of	Completion	
BH1	0.0 – 0.15	0.15 – 2.5	Terminated @ 2.5m
BH2	0.0 – 0.20	0.20 – 1.5	Terminated @ 1.5m
BH3	0.0 - 0.20	0.20 – 1.5	Terminated @ 1.5m
BH4	0.0 – 0.15	0.15 – 1.5	Terminated @ 1.5m
BH5	0.0 – 0.15	0.15 – 1.5	Terminated @ 1.5m
BH6	0.0 – 0.15	0.15 – 1.5	Terminated @ 1.5m
BH7	0.0 - 0.2	0.2 – 1.5	Terminated @ 1.5m
BH8	0.0 – 0.3	0.3 – 2.5	Terminated @ 3.0m

4.4 DYNAMIC CONE PENETROMETER (DCP) TEST RESULTS

Dynamic Cone Penetrometer (DCP) tests were performed adjacent to each test location, and advanced from the existing surface level. The DCP results are presented below:

DCP	DCP1	DCP2	DCP3	DCP4	DCP5	DCP6	DCP7	DCP8
Location	BH1	BH2	BH3	BH4	BH5	BH6	BH7	BH8
Datum				Existing Su	Irface Level			
Depth (m)			[OCP Blows/100	mm Penetratio	n		
0.0 – 0.1	3	2	2	2	2	6	1	1
0.1 – 0.2	4	2	3	2	2	7	2	2
0.2 – 0.3	3	2	2	2	2	6	2	2
0.3 – 0.4	2	2	2	2	2	6	3	2
0.4 – 0.5	2	2	3	2	2	5	2	3
0.5 – 0.6	3	2	3	2	1	5	2	3
0.6 – 0.7	3	2	3	2	1	5	2	3
0.7 – 0.8	3	2	5	2	1	5	3	3
0.8 – 0.9	-	2	-	2	1	-	-	-
0.9 – 1.0	-	2	-	2	1	-	-	-
1.0 – 1.1	-	4	-	3	1	-	-	-
1.1 – 1.2	-	4	-	7	2	-	-	-
1.2 – 1.3	-	7	-	7	2	-	-	-
1.3 – 1.4	-	-	-	-	4	-	-	-
1.4 – 1.5	-	-	-	-	6	-	-	-

Table 2: Dynamic Cone Penetrometer (DCP) Test Results

The DCP test results were observed to vary across test locations and with depth at each test position. The DCP tests generally indicate firm to stiff or better soil conditions at most test locations, however soft to firm test positions are noted to existing within several test positions. The DCP results indicate that poor site trafficability and workability of the soils will be experienced during wet winter months. Some difficulty may also be experienced in achieving proof rolling or compaction and it is strongly recommended that works commence during dry, summer months. Lower DCP blow counts are expected under elevated moisture levels.

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4.5 LABORATORY TEST RESULTS

Soil samples were recovered from soil units Unit 2 and Unit 3 at selected locations and soil horizons. Laboratory test reports are presented in Appendix C and summarised below:

Sample #	Location/Depth	NMC (%)	wL (%)	pL (%)	PI (%)	PI x %≤ 425µm
#1	BH2 (0.3 – 0.5)	21.7	40	20	20	1880
#3	BH2 (1.3 – 1.5)	17.1	52	25	27	2322
#5	BH5 (0.8 – 1.0)	21.7	70	25	45	4275

Table 3: Summary of Laboratory Tests

NMC: Natural Moisture Content; wL: Liquid Limit; pL: Plastic Limit; PI: Plasticity Index

The laboratory test results generally indicate the samples to comprise of medium to high plasticity clay soils, with varying fractions of sand and gravel noted, as observed in the boreholes. In general, the soil samples were noted to be either close to or dry of the inferred optimum moisture content. The Weighted Plasticity Index (PI x $%\leq425\mu$ m) indicates the Unit 2 soils on this site to be very highly expansive in nature.

5. DISCUSSION

The boreholes have shown a relatively consistent subsurface soil profile, comprising of up to 0.3m of Unit 1 topsoil overlying Unit 2 residual Humevale Formation deposits. The laboratory test results generally supported the findings of the boreholes and indicate the soils on this site to be very highly expansive (i.e. reactive and sensitive to volumetric changes under different moisture levels). The DCP test results indicate soft to firm soil zones at some test locations and a key geotechnical risk identified on this site is construction timing, which should occur during dry, summer months.

Careful consideration must also be given to the existing embankments or surrounding elevated areas, which may direct overland surface flows towards the field. Cut-off/perimeter drains and associated collector pits shall form part of the works in such areas. If water is allowed to pond against the edge of the synthetic field, differential movements are likely to be seen in the surface. The presence of rows of large trees is also considered a geotechnical risk on this project and the use of tree root protection systems is recommended, should the playing field edges be located within a distance of 1 x Mature Heights of the trees.

For the purposes of this report, it is assumed that the proposed synthetic playing field will comprise of a horizontally draining profile. Should a vertically draining profile be adopted, further advice shall be sought from Ground Science.

5.1 SUBGRADE CHARACTERISTICS

The proposed synthetic field may be suitably founded on a prepared subgrade of the Unit 2 soils. The Unit 1 topsoil shall form part of stripping works, which we anticipate to require between 0.2m and 0.3m of existing materials. Total stripping depths shall be assessed at the time of construction. The following procedures are recommended for the subgrade preparation phase of the project:

- All existing grass cover/topsoil/organic material/exposed silts or unsuitable materials are removed to
 expose the Unit 2 soils;
- An assessment of the condition of the exposed subgrade is required by this office or a qualified geotechnical practitioner;
- The subgrade shall be ripped/tyned and moisture conditioned to a depth of not less than 300mm, and to within +/- 3% of OMC, and subsequently compacted to achieve a target density ratio of not less than 98% Standard;



- If the above target density ratio is not achieved, further advice must be sought from Ground Science;
- A minimum of 6 field density tests is required by a suitably qualified NATA accredited testing body.
- The stripped subgrade prior must be proof rolled using a fully loaded water cart (or plant with similar axle loading characteristics with an equivalent load of 12 tonnes) with any soft spots, deflections or rutting to be further stripped until a firm base is achieved or suitably remediated as directed by the GITA;
- If applicable, any filling works must be undertaken to Level 1 procedures as described in AS3798 (2007) 'Guidelines on Earthworks for Commercial and Residential Developments' with compaction levels to be verified by field density testing undertaken by a NATA accredited testing facility;
 - Fill materials shall be free of organic matter, oversize particles including no particle over 20% by volume, particles coarser than 37.5mm and no particle over 200mm in any dimension;
 - Fill materials shall be placed in layers not exceeding 200mm loose thickness and compacted to achieve a target dry density ratio of not less than 95% Standard Compaction (AS1289 5.1.1, 5.4.1 or 5.7.1) and not exceeding 102% Standard Compaction;
 - The top 300mm of the finished subgrade level shall be compacted to achieve a target dry density ratio of not less than 98% Standard Compaction (and not more than 102% Standard Compaction) with a minimum test frequency of 6 tests required at the top of subgrade;
 - All fill materials shall be moisture conditioned to within +/- 3% of the Optimum Moisture Content (SOMC).

If service trenches are identified during the stripping phase, further DCP or field density testing is recommended, as pre-existing poorly backfilled service trenches will be detrimental to the performance of the future sports field. If further inspections identify poorly backfilled shallow trenches (<1.5m), this material may be replaced with suitable fill or stabilised sand. For deep service trenches, a more specific remediation method may be adopted, such as the use of geotextiles or similar. This shall form part of further assessments after site stripping is completed. It is further recommended that if deleterious materials, organics, silts or unstable soils are encountered during the stripping phase, these materials shall also be removed.

5.2 SUBGRADE PROTECTION

Given the reactive and expansive nature of the subgrade, a uniform subgrade protection layer must be included in the design. The subgrade protection layer shall comprise a VicRoads Type A (soft rock) capping material and be a minimum 200mm thick. Alternative products may also be considered, such as a 40mm minus NDCR or a 20mm VicRoads Class 4 FCR.

Alternative products (to a VicRoads Type A soft rock) must be first tested for compliance to the below selection criteria:

	Physical Propert	ies	Lir	nits of Gradi	ng	PI x %>425µm	PI
CBR %	Swell %	Permeability m/s	40mm	4.75mm	75µm	<1000	< 25
≥6	≤1.5	≤5 x 10 ⁻⁹	100	60-80	10-40	1000	=20

Table 4: Capping Material Selection Criteria

It is advised that the selection of an appropriate capping material be undertaken during early stages of the project. Suitable capping materials, complying with Table 4 above, can be difficult to source in the current industry. The use of subgrade stabilisation (using lime/cement) may be considered as an alternative solution to the capping layer. If adopted, further advice should be sought and further optimum lime content testing may be warranted.

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The capping layer shall be compacted to achieve a target density ratio of not less than 98% Standard Compaction with a minimum of 6 field density tests performed. The capping layer must extend outside the boundary of field by a minimum of 1.5m to prevent moisture infiltration into the pavement edges. The capping layer shall be finished with a smooth drum roller to prevent any protruding angular rocks or sharp edges.

A composite membrane shall be placed over the capping layer with all joins machine welded. The membrane shall also extend 1.5m outside the edge of the field (including aprons/footpaths) with the edges turned down or suitably anchored. The composite membrane must be an approved single product consisting of two layers of A24 grade geofabric or similar either side of a polyethylene membrane. The composite geomembrane must be between $0.2\mu m$ and $0.7\mu m$ in thickness.

It is further recommended that root barriers are installed along all court perimeters located within a distance of 1 x Mature Tree Heights. If trees are removed, tree roots/stumps must be fully grubbed and the soils within this zone allowed to achieve equilibrium or alternatively suitably moisture conditioned and recompacted as part of the bulk earthworks program.

5.2.1 SURFACE PROFILE

The proposed sportsfield profile may comprise a minimum 175mm thick VicRoads 20mm Class 2 or Class 3 FCR base course, compacted to achieve a target density ratio of not less than 98% Modified Compaction. The proposed playing surface may be subsequently installed over the base layer and is subject to the designer's specification.

Consideration must be given to the application of corking at joints between asphalt and concrete (including spoon drains or footpaths) around the site, if applicable. Cracks that form at this interface may exacerbate the potential for moisture infiltration into lower lying layers of the profile. A regular review and maintenance period are required of all joints around the playing field, with any observed separation be suitably remediated.

5.2.2 EFFECTS OF TREES & DRAINAGE

A suitably designed drainage system is considered critical on sports fields. We recommend that service trenches are avoided below the field, however should this not be possible, all subsurface trenches must be suitably lined with the composite membrane. Common failures in synthetic fields are a result of poor backfill procedures of all service trench works. It is highly recommended that all backfill procedures for service trench work is tested for compaction or work method placements validated by a qualified geotechnical engineer. We recommend hold points are attached to these functions for all works under critical structures.

Suitably designed spoon drains should form part of the design with the subgrade protection layer to extend past the concrete spoon drains. Consideration must also be given to drainage of surrounding areas, particularly where the existing batters slope towards the courts. It is recommended that edge batters or slopes should incorporate suitable cut of drains to ensure no edge infiltration of moisture occurs at any field perimeter.

Due to the reactive natural clay subgrade conditions in some areas, trees within close proximity to the playing courts will likely result in further shrinking and swelling of the subgrade soils, causing differential movements and pavement failure. Due to the fine tolerances within synthetic fields it is suggested that root barriers are installed as previously recommended in this report. A qualified arborist should be commissioned to provide advice regarding mature heights and safe distances. Careful consideration should be given to the removal of trees, as tree removal may allow for the subgrade soils to become wet and swell, potentially compromising the performance of the pavements.

Drainage during the construction phase of the works must also be suitably managed. The timing of construction, levels of stripping and delays from the stripping phase to construction phase are all likely to contribute to further issues, if not suitably managed. All areas of the site must be suitably drained and/or graded to prevent ponding

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of surface water. Dry weather periods are highly likely to result in surface desiccation of the reactive clays and if left exposed, may act as a conduit for water to penetrate the lower lying subgrade soils.

6. CLOSURE

Ground Science must be consulted and confirmation of the applicable site conditions, design procedures, construction/material specifications and proposed methodology will require a review, depending on the choice of works. We appreciate that different construction companies may adopt different construction approaches including (but not limited to) material sourcing, equipment, timeframes, methodology and required hold points. Sufficient time and cost shall be allowed for by the successful tenderer to open discussions with Ground Science prior to construction.

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For & on behalf of Ground Science Pty Ltd

Gee Singh

Gee Singh Senior Geotechnical Engineer



7. LIMITATIONS

The advice provided in this document (as per our commission) is not designed or capable of identifying all soil conditions, (which can vary with products chosen). The advice given in this document is based on the assumption that the test results are representative of the overall soil conditions. However, it should be noted that actual conditions in some parts of the site might differ from those found. If further sampling/ testing reveals soil characteristics significantly different from those shown in our findings, Ground Science must be consulted.

The scope and the period of Ground Science services are described in the document and are subject to restrictions and limitations. Ground Science did not perform a complete assessment of all possible conditions or circumstances that may exist. If a service is not expressly indicated, do not assume it has been provided. If a matter is not addressed, do not assume that any determination has been made by Ground Science in regards to it.

Where data has been supplied by the client or a third party, it is assumed that the information is correct unless otherwise stated. No responsibility is accepted by Ground Science for incomplete or inaccurate data supplied by others.

It is recognised that the passage of time affects the information and assessment provided in this document. Ground Science's assessment is based on information that existed at the time of the preparation of this document. It is understood that the services provided allowed Ground Science to form no more than an opinion of the actual site conditions observed during sampling and observations of the site visit and cannot be used to assess the effects of any subsequent changes in the quality of the site, or its surroundings, or any laws or regulations.

Any drawings or figures presented in this report should be considered only as pictorial evidence of our work. Therefore, unless otherwise stated, any dimensions should not be used for accurate calculations or dimensioning.

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8. REFERENCES

- AS1289 Testing of Soils for Engineering Purposes.
- Geological Survey of Victoria, Ringwood Map Sheet (scale 1:63,360)
- AS3798 2007 Guidelines on Earthworks for Residential and Commercial Developments.
- Austroads 2017 Guide to Pavement Technology, Part 2: Pavement Structural Design.

APPENDIX A

Figure 2: Site Locality Plan



APPENDIX B

Borehole Log Sheets

	GroundScien	CE ENGINEERING BOREHOLE LOG	Borehole No JOB No :	BH1 64091.1
CLIENT: S PROJECT: E LOCATION: S TEST LOCATION: R	SPORTENG Pty Ltd Benedikt Reserve Scoresby Refer to Site Plan, Appendix A		TEST DATE: LOGGED BY: CHECKED BY:	30-Oct-19 SE GS
DRILL METHOD: Hole Diameter:	ATS Drill Rig 100mm	EASTING: ND NORTHING: ND	INCLINATION: SURFACE RL:	90° ND
DRILLING	SAMPLING	FIELD MATERIAL DESCRI	PTION E	1
C PENERTRATION C RESISTANCE MATER DEPTH (metres)	DEPTH (RL) SAMPLE OR FIELD TE RECOVERED	SOIL / ROCK MATERIAL DESCRIPTION	CONSISTENCY DENSI MOISTURE	ADDITIONAL OBSERVATIONS
0.0		CL FILL: silty CLAY, low plasticity, pale brown, with rootlets, trace sand	St D	Topsoil
	1.30	CH sity CLAY, high plasticity, brown motified dark grey, gravel inclusions U U	F - SI D-Dp St - VSI D	Inferred Anderson Creek Formation
2.5	2.50	Borehole Terminated @ 2.5m		
30				
PENETRATION	CONSISTENCY Vs Very	DENSITY MOISTURE CONDITION TEST NOTES Soft Fb Friable D Dry PP Pocks	et Penetrometer Test	Groundwater Level
no resistence	S Soft F Firm St Stiff VSt Very H Hard	VL Very Loose Dp Damp U63 Undit L Loose M Moist D Distu MD Medium Dense W Wet Bs Bulk: D Dense S Saturated E Envin VD Very Dense PL Plastic Limit HSV Hand LL Liquid Limit Cu Undra	suroed Sample 63mm rbed Sample Sample onmental sample Shear Vane test ained Shear Strength	UTP Unable to Penetrate

Q								ENGINEERING BOREHOLE LO	G	Borehole	No	BH2 G4091.1	2	
CLIENT: PROJECT: LOCATION: TEST LOCA	.TION:		SPORTENG Benedikt Res Scoresby Refer to Site	Pty Ltd erve Plan, Appendix A						TEST DAT LOGGED CHECKEE	TE: BY:) BY:	30-Oct-19 SE GS		
DRILL METH HOLE DIAM	HOD: ETER:		ATS Drill Ri 100mm	g				EASTING: ND NORTHING: ND		INCLINAT	10N: : RL:	90° ND		
	DRI	LLING	1	SAMPLING	ì			FIELD MATERIAL DESC	CRIPTION	Ł	1	1		
2 PENERTRATION C RESISTANCE	4 WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TE	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL / ROCK MATERIAL DESCRIPTION		CONSISTENCY DENSI	MOISTURE	ADD	ITIONAL OBSERVATIONS	\$
		0.0	-			00	CL-CI	FILL: silly CLAY, low to medium plasticity, dark brown, with rootlets, trace sand		F	D		Topsoil	
			0.20			Ŭ	СН	silty CLAY, high plasticity, brown mottled grey		F	D-Dp	Inferred Ar	derson Creek Formation	-1
				#1	D	U U								
						Ŭ U								
		0.5	0.40			U U								
			0.00			Ŭ		рае отомп полесь у сумплетес						
				#2	D	U U								13
						Ŭ U								
		1.0	1.00			U				St - VSt	D			
						Ŭ U								
				#3	D	U U								13
			1.40			Ŭ		with sand						
		1.5	1.50					Borehole Terminated @ 1.5m						
														=
			-											
		2.0												
														13
		2.5												
														13
			-]
		3.0	-											
]
		-	-											
]
PENETRATI	ION	3.5	1	CONSISTENCY				DENSITY MOISTURE CONDITION TEST NOTES						
	1 2	3 4		Vs S	Very Soft	Soft		Fb Friable D Dry PP Pc VL Very Loose Dp Damp U63 Ur	ocket Penetrome ndisturbed Sami	eter Test ble 63mm	n	UTP U	roundwater Level nable to Penetrate	
ľ		L	6	F	Firm			L Loose M Moist D Di MD Medium Dense W Wet Rs Br	sturbed Sample					
no r	esistenci reli	İsal		VSt H	Very Hard	Stiff		D Dense S Saturated E Er VD Very Dense PL Plastic Limit HSV Ha	nvironmental sar and Shear Vane	nple test				
HAL	1100	2.0.2000						LL Liquid Limit Cu Ur	ndrained Shear	Strength			Chool 1 of	1

									ENGINI	EERIN	G B(OREHO	DLE L	.0G	Borehole	No	<i>BH</i> G4091.1	13	
CLIENT: PROJECT: LOCATION: TEST LOCA	TION:		SPORTENG Benedikt Res Scoresby Refer to Site I	Pty Ltd erve Plan, Appendix A											TEST DA LOGGED CHECKE	TE: BY: D BY:	30-Oct-14 SE GS	9	
DRILL METH	iod: eter:		ATS Drill Ri 100mm	9				EASTING: NORTHING	ND 6: ND						INCLINAT	fion: E RL:	90° ND		
	DRII	LLING	-	SAMPLING	3		r					FIELD	MATERIAL	DESCRIPTION			1		
C PENERTRATION C RESISTANCE	4 WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TES	RECOVERED	GRAPHIC LOG	USC SYMBOL			SOIL / ROCK I	MATERIA	L DESCRIPTION			CONSISTENCY DENSIT	MOISTURE		ADDITIONAL OBSERVATIONS	5
		0.0	-			00	CL-CI	FILL: silty C	LAY, low to medium pla	asticity, dark browr	n, with roo	tlets, trace sand			F - St	D		Topsoil	
		-	0.20			Ö	СН	silty CLAY,	high plasticity, brown m	ottled grey, gravel	inclusions	5			F	D-Dp	Inferr	ed Anderson Creek Formation	
						Ũ U													
		-	0.40			U		pale brown	mottled grey/white/red						St - VSI				
		0.5				Ŭ U													
		-				U													
			0.80			Ŭ										D	-		
		-				U													
		1.0				Ŭ													
		-				Ŭ													
		-	-			U													
		-				Ŭ													
	+	1.5	1.50			U	-	Borehole Te	erminated @ 1.5m										
		-																	
		-																	
		-	-																
		2.0																	\square
		-																	
		_	-																
		-																	
		- 25	-																
		-																	
		_																	13
		-																	
		-																	
		3.0																	13
		-																	
		-																	
PENETRATI	ON	3.5	1	CONSISTENCY				DENSITY		MOIST	URE CON	IDITION	TEST NOT	ES					
	1 2	3 4		Vs S	Very Soft	Soft		Fb VL	Friable Very Loose	D Do		Dry Damp	PP U63	Pocket Penetrom Undisturbed Sam	ieter Test iple 63mr	n	UTP	Groundwater Level Unable to Penetrate	
		L	4	F St	Firm			L MD	Loose Medium Dense	M	I	Moist Wet	D Bs	Disturbed Sample Bulk Sample	e				
no re	esistence relu	isal		VSt H	Very	Stiff		D	Dense Very Dense	S		Saturated Plastic Limit	E HSV	Environmental sa Hand Shear Van	ample e test				
LIAL	1100	2.0.2000			, iaiu					LL		Liquid Limit	Cu	Undrained Shear	Strength			Chaol 1 of	1

U		Gro	ENGINEERING BOREHOLE LOG)LE LOG	Borehole JOB No :	No	BH4 G4091.1				
CLIENT: PROJECT: LOCATION: TEST LOCATIO	DN:	SPORTENG Benedikt Res Scoresby Refer to Site	Pty Ltd verve Plan, Appendix A							TEST DA LOGGED CHECKE	TE: BY: D BY:	30-Oct-19 SE GS	
DRILL METHO HOLE DIAMET	D: ER:	ATS Drill Ri 100mm	g				EASTING: ND NORTHING: ND			INCLINAT SURFAC	TION: E RL:	90° ND	
	DRILLING	1	SAMPLING	; T				FIELD	MATERIAL DESCRIPTION	L			
2 PENERTRATION 5 RESISTANCE 4	WATER DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TE	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL	/ ROCK MATERIAL DESCRIPTION		CONSISTENCY DENSI	MOISTURE	ADDITIONAL OBSERVATION	IS
	0.0	-			000	CL-CI	FILL: silty CLAY, low to medium plasticity, d	ark brown, with rootlets, trace sand		F	D	Topsoil	
	0.5	0.15				СН	silly CLAY, high plasticity, brown motified gro	ey, trace rootlets and gravel inclusions	5	F	Dp	Inferred Anderson Creek Formation	
	-	- - - - - - 0.90					pale brown motiled grey/whitehed						
	-	1.10 - 1.10 								St - VSI	D		
	1.5	- 1.50 					Borehole Terminated @ 1.5m						
	3.0												
PENETRATION	3.5	1	CONSISTENCY				DENSITY	MOISTURE CONDITION	TEST NOTES				
no resis	1 2 3 4		Vs S F St VSt H	Very Soft Firm Stiff Very Hard	Soft		Fb Friable VL Very Loose L Loose MD Medium Dense D Dense VD Very Dense	D Dry Dp Damp M Moist W Wet S Saturated PL Plastic Limit LL Liquid Limit	PP Pocket Penetro U63 Undisturbed Sa D Disturbed Samp Bs Bulk Sample E Environmental s HSV Hand Shear Va Cu Undrained Shea	meter Test mple 63mr ble sample ne test ar Strength	n	Groundwater Level UTP Unable to Penetrate	(1

	U								ENGINEER	RING BOREHO	DLE LOG	Borehole JOB No :	No	BH5 G4091.1	
CLIEN PROJI LOCA TEST	T: ECT: TION: LOCATI	ION:		SPORTENG Benedikt Res Scoresby Refer to Site	Pty Ltd erve Plan, Appendix A							TEST DA LOGGED CHECKEI	TE: BY: D BY:	30-Oct-19 SE GS	
DRILL HOLE	METHO DIAME	DD: TER:		ATS Drill Ri 100mm	9				EASTING: ND NORTHING: ND			INCLINAT	TION: E RL:	90° ND	
		DRIL	LING	1	SAMPLING	;		1	1	FIELD	MATERIAL DESCRIPTION	Σ		I	
1 DENERTRATION	⊷ RESISTANCE	AMATER WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TE	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL /	ROCK MATERIAL DESCRIPTION		CONSISTENCY DENSI	MOISTURE	ADDITIONAL OBSER	VATIONS
			0.0				Q	CL-CI	FILL: silty CLAY, low to medium plasticity, da	rk brown, with rootlets, trace sand		F	D	Topsoil	
			-	0.15			Ŭ	СН	silty CLAY, high plasticity, brown mottled gre	y, trace rootlets and gravel inclusions	5	F	Dp	Inferred Anderson Creek For	mation
							Ŭ								
			-		#4	D	Ü								
			0.5				Ŭ								
			-				Ŭ								
			-	-	#5	D	U U								
			-				Ŭ								
			1.0	1.00			Ŭ		brown mottled grey/red						
			-				Ŭ								
	Ш		-	1.20			Ü					St. VSt	D Dn	-	
	Ш		-	1.30			Ŭ					31- 131	. о - ор		
H	╂╂	┢	1.5	1.50		+	Ū		Borehole Terminated @ 1.5m						-+-
			-	-											
			-												
			-	-											
			2.0												H
			-												
			-												
			-												
			2.5												
			-												
			-												
			2.0]
			3.0												
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PENE	IRATIO	N	3.5	L	CONSISTENCY			1	DENSITY	MOISTURE CONDITION	TEST NOTES		1	l	
	~	1 2	3 4		Vs S	Very Soft	Soft		Fb Friable VL Very Loose	D Dry Dp Damp	PP Pocket Penetron U63 Undisturbed San	neter Test nple 63mr	n	Groundwater Le UTP Unable to Penet	vel rate
			<u>ل</u>	5	F St	Firm Stiff			L Loose MD Medium Dense	M Moist W Wet	D Disturbed Sampl Bs Bulk Sample	e			
1	no res	istence refus	sal		VSt H	Very Harc	Stiff 1		D Dense VD Very Dense	S Saturated PL Plastic Limit	E Environmental sa HSV Hand Shear Van	ample e test			
L	ЦАМ	100.1	0.0000						l	LL Liquid Limit	Cu Undrained Shea	Strength		Sha	of 1 of 1

U							•	ENGINEERING BOREHOLE LOG			Borehole JOB No :	No	BH G4091.1	6	
CLIENT: PROJECT: LOCATION: TEST LOCAT	10N:	S E S	SPORTENG Benedikt Res Scoresby Refer to Site I	Pty Ltd erve Plan, Appendix A							TEST DA LOGGED CHECKE	TE: BY: D BY:	30-Oct-19 SE GS		
DRILL METH	DD: TER:		ATS Drill Ri 100mm	9				EASTING: ND NORTHING: ND			INCLINAT SURFACI	fion: E rl:	90° ND		
	DRILI	LING		SAMPLING	3		1		FIELD	MATERIAL DESCRIPTION	≥		1		
2 PENERTRATION 5 RESISTANCE	4 WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TE	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL	L / ROCK MATERIAL DESCRIPTION		CONSISTENCY DENSI	MOISTURE	AE	DDITIONAL OBSERVATIONS	;
		0.0				00	CL-CI	FILL: silty CLAY, low to medium plasticity, p	pale brown, with rootlets, trace sand		VSt	D		Tapsoil	
		-	0.15				СН	silty CLAY, high plasticity, brown mottled g	rey, trace rootlets		VSt	D	Inferred	Anderson Creek Formation	
		0.5	0.50			U U U U		brown/red mottled grey							
		-													
		-				U U U									
		1.0				Ŭ U U U									
		-	1.30			U U U U		brown mottled grey/white/red			St - VSI				
	+	1.5	1.50			U		Borehole Terminated @ 1.5m							
		-													
		20													
		-													
		-													
		2.5													
		-													
		3.0													
		-													
		-													
PENETRATIC	DN	3.5	1	CONSISTENCY				DENSITY	MOISTURE CONDITION	TEST NOTES					
) no res	1 2	3 4		Vs S F St VSt H	Very Soft Firm Stiff Very Hard	Soft		Fb Friable VL Very Loose L Loose MD Medium Dense D Dense VD Very Dense	D Dry Dp Damp M Moist W Wet S Saturated PL Plastic Limit	PP Pocket Penetrr U63 Undisturbed Sa D Disturbed Sam Bs Bulk Sample E Environmental HSV Hand Shear Va	ometer Test ample 63mr ple sample ane test	n	UTP	Groundwater Level Unable to Penetrate	
НАМ	106.2	0.2000							LL Liquid Limit	Cu Undrained She	ar Strength			Shoot 1 of	1

			Gro	undSa	ien	ice	9	ENGINEE	RING BOREHO)LE LOG	Borehole JOB No :	No	BH7	
CLIENT: PROJECT	:		SPORTENG Benedikt Res Scoresby	Pty Ltd serve							TEST DA LOGGED CHECKE	TE: BY: D BY:	30-Oct-19 SE GS	
DRILL ME	THOD:		ATS Drill Ri	Plan, Appendix A				EASTING: ND NORTHING: ND			INCLINAT	FION:	90°	
HOLE DIA	DI	RILLING	Tuomini	SAMPLING	S		1	1	FIELD	MATERIAL DESCRIPTION	JURFACI	L KL.	ND	
2 PENERTRATION C RESISTANCE	4	WATER DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL	/ ROCK MATERIAL DESCRIPTION		CONSISTENCY DENSIT	MOISTURE	ADDITIONAL OBSERVATIO	ONS
	Π	0.0	-			00	CL-CI	FILL: silty CLAY, low to medium plasticity, d	ark brown, with rootlets, trace sand		F	D	Topsoil	
			0.20			0 II	СН	silty CLAY, high plasticity, brown mottled gre	w.		F - St	Dp	Inferred Anderson Creek Formatio	0
		-	3			Ŭ U			,					
			-			U U								
		0.5	-			Ŭ								
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		-	-			U U								
			-			Ŭ								
		1.0	-			Ŭ U								H
			-			U								
		-	-			Ŭ								
			1.30			Ŭ U		brown mottled grey/white/red			St - VSI	D		
		1.5	1.50			U U		Rorehole Terminated @ 1.5m						
			-											
			3											
			1											
			-											
		2.0	-											
			1											
		-	-											
			-											
		2.5	-											
]											
		-	-											
			-											
		3.0	-											
			1											
		-	4											
			3]
		3.5	4											
PENETRA	TION	1 -	1	CONSISTENCY	Von	Soft	•	DENSITY		TEST NOTES	motor Tool	•	Groundwater Level	
	1 ≥∎	234	1	S	Soft	JUIL		VL Very Loose	Dp Damp	U63 Undisturbed Sa	imple 63mr	n	UTP Unable to Penetrate	
		<u> </u>	k,	St	r irm Stiff	.		MD Medium Dense	W Wet	Bs Bulk Sample	ue			
no	resister P	nçe refusal		VSt H	Very Hard	Stiff		D Dense VD Very Dense	S Saturated PL Plastic Limit	E Environmental HSV Hand Shear Va	sample ine test			
L.,	MIO	C 2.0.2000)					1	LL Liquid Limit	Cu Undrained She	ar Strength		Chool 1	of 1

			Gro	undSc	ien	ice	9	ENGINEER	ING BOREHO	DLE LOG	Borehole JOB No :	No	BH8 G4091.1		
CLIENT: PROJECT: LOCATION: TEST LOCA	TION:		SPORTENG Benedikt Res Scoresby Refer to Site	Pty Ltd verve Plan, Appendix A							TEST DA LOGGED CHECKEI	TE: BY:) BY:	30-Oct-19 SE GS		
DRILL MET HOLE DIAN	HOD: Ieter:		ATS Drill Ri 100mm	g				EASTING: ND NORTHING: ND			INCLINAT SURFACE	10N: E RL:	90° ND		
	DRII	LLING		SAMPLING	S				FIELD	MATERIAL DESCRIPTION	È	1	1		
 C C D D E B E S /ul>	4 WATER	DEPTH (metres)	DEPTH (RL)	SAMPLE OR FIELD TE	RECOVERED	GRAPHIC LOG	USC SYMBOL	SOIL / R	OCK MATERIAL DESCRIPTION		CONSISTENCY DENS	MOISTURE	ADDIT	IONAL OBSERVATIONS	;
		0.0	-			00	CL	FILL: silty CLAY, low plasticity, pale brown, with	h rootlets, trace sand		F - St	D		Topsoil	
						ğ									E
			0.30			0 U	СН	silty CLAY, high plasticity, brown mottled dark	grey		F - St	Dp	Inferred And	erson Creek Formation	+ -
		.	-			Ŭ									
		0.5				Ŭ									
		.	0.60			U		brown mottled grey/red							
			-			Ŭ									
						U									
		1.0				Ŭ									
						Ŭ U									
		-	-			Ü									
			-			Ŭ									
		1.5				U									Н
		.	1.60			Ŭ		white/brown			St - VSt	D	-		
		.				Ŭ									1-
		.	-			Ŭ									
		2.0				Ŭ U									H
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	\parallel	2.5	2.50			Ŭ		Borehole Terminated @ 2.5m							┤╶┨
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		3.0													=
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		.	-												
PENETRAT	ION	3.5	I	CONSISTENCY	1		l	DENSITY	IOISTURE CONDITION	TEST NOTES		l	<u> </u>		
	1 2	3 4		Vs S	Very Soft	Soft		Fb Friable VL Very Loose	D Dry Dp Damp	PP Pocket Penetro U63 Undisturbed Sa	meter Test mple 63mr	n	UTP Una	undwater Level ble to Penetrate	
		Ĺ	4	F St	Firm Stiff			L Loose MD Medium Dense	M Moist W Wet	D Disturbed Samp Bs Bulk Sample	le				
nor	esistence relu	isal		VSt H	Very Hard	Stiff		D Dense VD Very Dense	S Saturated PL Plastic Limit	E Environmental s HSV Hand Shear Va	ample ne test				
HA	4106	2.0.2000							LL Liquid Limit	Cu Undrained Shea	ar Strength			Shoot 1 of	1

APPENDIX C

Laboratory Test Results


Particle Size Distribution & Atterberg Limits Test Report

A C N 105 704 078

13 Brock Street, Thomastown, VIC P 03 9464 4617 Email reception@groundscience.com.au

Client:	SPORTENG PTY	LTD				Job No.	G4091.1	
Project:	BENEDIKT RESE	RVF				Test Date:	18-Nov-19	
Logetion:	SCODESDV					Poport No	A A	
Location.	No	#1	Samplo	Idontification			AA	
Lab Reference	; NO.	<i>#</i> 1	Cample	identification.		вна @ 0.3 - 0.5		
Laboratory Spo	ecimen Classific	ation:	CLAY, me	dium plasticity, yel	low/b	rown, trace sand	, trace gravel	
Particle Size I	Distribution	AS1289 3.6.1	Consis	tency Limits a	Ind I	Moisture Cor	itent	
Sieve Size	% Passing	Specification		Test		Method	Result	Spec.
150 mm	100		Liquid L	_imit	%	AS1289 3.1.2	40	
75 mm	100		Plastic	Limit	%	AS1289 3.2.1	20	
53mm	100		Plastici	ty Index	%	AS1289 3.3.1	20	
37.5 mm	100		Linear S	Shrinkage	%	AS1289 3.4.1	7.0	
26.5 mm	100		Moistur	e Content	%	AS1289 2.1.1	21.7	
19.0 mm	100		Sample H	listory:			Oven Dried	
13.2 mm	100		Preparatio	on Method:			Dry sieved	
9.5 mm	100		Crumbling	g / Curling of linear	shrin	kage:	Cracking	
6.7 mm	99		Linear sh	rinkage mould leng	th:		250 mm	
4.75 mm	99			ND = not determin	led	NO = not obtain	able NP = non p	lastic
2.36 mm	96		Moisture	/ Dry Density Rela	ations	ship	AS 1289 5.2.1	
1.18 mm	95		Maximum	Dry Density:			t/m ³	
600 um	95		Optimum	Moisture Content:			%	
425 um	94							
300 um	94		Notes:	Sampling Metho	bd	AS1289 1.2.1(6.	5.3)	
150 um	93				Sam	pled by SE 30/10	0/2019	
75 um	92							
		Р	Particle Size	e Distribution	Soil clas	sification referenced from 1	Table 1 A51726	A.S.
100			5 150	300 425 600 1	.18	2,36 4,75 9	.5 13,2 19 26.5 37.5	5 63
90								
80								
70								
_ /0								
0 issi								
<u>م</u> 50 – <u>ا</u>								
eue 40								
30								
50								
20								
10								
0								
clay	o.o1		U.I Pari	ticle Size (mm) ¹ sand		ar	avel	100
TECHNICAL	NATA Accredited Accredited for cor The results of the included in this do Standards	Laboratory No. 15055 mpliance with ISO/IEC tests, calibrations and ocument are traceable	17025 - Test I/or measure to Australiar	D ments n/National	Tim App	21/11/2019 Senserrick proved Signate	I -I ory	

GS005A/R V9 Oct 2018 App KC



Particle Size Distribution & Atterberg Limits Test Report

A C N 105 704 078

13 Brock Street, Thomastown, VIC P 03 9464 4617 Email reception@groundscience.com.au

Client:	SPORTENG PTY	LTD			Job No.	G4091.1	
Project:	BENEDIKT RESE				Test Date:	18-Nov-19	
					Depart No.	AD	
Location:	SCORESBY	#2	Commission	al a satisfi a a ti a sa s	Report No.	AB	
Lab Reference	NO.	#3	Sample	identification:	BH2 @ 1.3 - 1.5	m	
Laboratory Spe	ecimen Classific	ation:	CLAY, high	n plasticity, light brown,	trace gravel, trac	e sand.	
Particle Size	Distribution	AS1289 3.6.1	Consist	tency Limits and	Moisture Cor	itent	
Sieve Size	% Passing	Specification		Test	Method	Result	Spec.
150 mm	100		Liquid L	imit %	AS1289 3.1.2	52	
75 mm	100		Plastic L	_imit %	AS1289 3.2.1	25	
53mm	100		Plasticit	y Index %	AS1289 3.3.1	27	
37.5 mm	100		Linear S	Shrinkage %	AS1289 3.4.1	4.0	
26.5 mm	100		Moisture	e Content %	AS1289 2.1.1	17.1	
19.0 mm	100		Sample Hi	istory:		Oven Dried	
13.2 mm	100		Preparatio	n Method:		Dry sieved	
9.5 mm	97		Crumbling	/ Curling of linear shri	nkage:	Cracking	
6.7 mm	92		Linear shr	INKage mould length:		250 mm	
4.75 mm	91			ND = not determined	NO = not obtain	able $NP = non p$	astic
2.36 mm	89		Moisture /	Dry Density Relation	ship	AS 1289 5.2.1	
1.18 mm	88		Maximum	Dry Density:		t/m ³	
600 um	87		Optimum I	Moisture Content:		%	
425 um	86						
300 um	86		Notes:	Sampling Method	AS1289 1.2.1(6	5.3)	
150 um	86			Sam	pled by SE 30/10	0/2019	
75 um	85						
		P	article Size	Distribution Soilda	silication referenced from 1	Table 1 AS1726	A.S.
100			150	300 425 600 1.18	2,36 4,75 9	.5 13,2 19 26.5 37.5	63
90							
80		•					
00							
70							
buiss 60							
4 50							
ceu							
^b 40							
30							
20							
10							
o							
0.001 clav	0.01 cilt		0.1 Parti	icle Size (mm) 1		10 0.40	100
Ciay	SIIL	1	S		u gr	avei	1
TROMMETENCE	NATA Accredited Accredited for cor The results of the included in this do Standards	Laboratory No. 15055 npliance with ISO/IEC tests, calibrations and ocument are traceable	17025 - Testii /or measurer to Australian	ng ments /National Tim Ap	Senserrick	bory	

GS005A/R V9 Oct 2018 App KC



Particle Size Distribution & Atterberg Limits Test Report

A C N 105 704 078

13 Brock Street, Thomastown, VIC P 03 9464 4617 Email reception@groundscience.com.au

Client:	SPORTENG PTY	LTD				Job No.	G4091.1	
Project:	BENEDIKT RESE	RVF				Test Date:	18-Nov-19	
	CODECDV					Dement Ma	10-110-13	
Location:	SCORESBY	#5	Comple	Idontificatio		Report No.	AC	
Lab Reference	e NO.	#5	Sample	Identificatio	n:	BH5 @ 0.8 - 1.0	Im	
Laboratory Spo	ecimen Classific	ation:	CLAY, hig	h plasticity, bro	own, trace	e sand, trace grav	vel.	
Particle Size I	Distribution	AS1289 3.6.1	Consis	tency Limit	s and I	Moisture Cor	ntent	
Sieve Size	% Passing	Specification		Test		Method	Result	Spec.
150 mm	100		Liquid L	.imit	%	AS1289 3.1.2	70	
75 mm	100		Plastic	Limit	%	AS1289 3.2.1	25	
53mm	100		Plasticit	ty Index	%	AS1289 3.3.1	45	
37.5 mm	100		Linear S	Shrinkage	%	AS1289 3.4.1	12.5	
26.5 mm	100		Moistur	e Content	%	AS1289 2.1.1	21.7	
19.0 mm	100		Sample H	listory:			Oven Dried	
13.2 mm	100		Preparatio	on Method:			Dry sieved	
9.5 mm	100		Crumbling	g / Curling of lir	hear shrin	kage:	-	
6.7 mm	99		Linear shi	rinkage mould	length:		-	
4.75 mm	98			up = not deter	rmined	INU = not obtain	able NP = non p	DIASTIC
2.36 mm	96		Moisture	Dry Density I	Relations	ship	AS 1289 5.2.1	
1.18 mm	95		Maximum	Dry Density:			t/m ³	
600 um	95		Optimum	Moisture Conte	ent:		%	
425 um	95							
300 um	95		Notes:	Sampling M	lethod	AS1289 1.2.1(6	.5.3)	
150 um	94				Sam	pled by SE 30/1	0/2019	
75 um	94							
		P	article Size	e Distribution	Soil class	sification referenced from	Table 1 AS1726	A.S.
100		75	150	300 425 600	1.18	2,36 4,75 9	0.5 13,2 19 26.5 37.	5 63
90								
80								
50								
70								
uissi 60								
G 50								
erce 40								
30								
20								
10								
0	0.01						10	100
clay	silt		v.i Part	ticle Size (mm) sand	I	ar	avel	100
TECHNICAL	NATA Accredited Accredited for cor The results of the included in this do Standards.	Laboratory No. 15055 npliance with ISO/IEC tests, calibrations and ocument are traceable	17025 - Testi /or measure to Australiar	ing ments n/National	Date: ^{Tim} App	21/11/2019 Senserrick proved Signat	ory	

GS005A/R V9 Oct 2018 App KC APPENDIX D

Site Photographs











INSIDE EDGE SPORT AND LEISURE PLANNING

PROPOSED EASTERN FOOTBALL HUB

WANTIRNA SOUTH RECREATION RESERVE

50 MOUNTAIN HIGHWAY

WANTIRNA

Report No: 115706

24 March 2014 Date:

GEOTECHNICAL INVESTIGATION (PRELIMINARY)

By

A.S. JAMES PTY LIMITED 15 Libbett Avenue, Clayton South Vic. 3169 Tel: 613 9547 4811 Fax: 613 9547 5393 E-mail: melb@asjames.com.au

THIS REPORT SHALL ONLY BE REPRODUCED IN FULL

X:James\Geotechnical\Commercial\115706 Eastern Football Hub, WANTIRNA\FINAL REPORT DOCUMENTS\115706 Proposed Eastern Football Hub, docx



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1. INTRODUCTION

- 1.01 <u>Investigation Requested By</u>: The geotechnical investigation was commissioned by Mr Michael Bodman Bolton of Inside Edge Sport and Leisure Planning via email correspondence dated 21st February 2014.
- 1.02 <u>Purpose of Investigation</u>: The scope for this investigation is as follows:

General: It was proposed to develop the Wantirna South Recreation Reserve into an Australian Rules Football training and administration hub. In doing so, bringing together training and development functions of the Eastern Football League, Eastern Rangers TAC, Eastern Region Umpires, AFL Victoria and Hawthorn Football Club.

Proposed Infrastructure: The proposed developments to the site are as follows:

- The conversion of the existing playing field (eastern oval) from grass to an AFL/CA approved synthetic oval
- The development of disused open space (current picnic/play area) to premier quality natural turf, sand based oval of MCG size and quality
- The development of a 1500m2 (double story) office administration, social and change room complex
- The development of roadways and car parking to meet the demands of the upgraded site

Information provided indicates the site was believed to be a landfill pre 1968 and the existing road ways were developed in 1998. The likely specifications for the proposed main natural grass playing field or (Western Oval) are in accordance with the '*Colac Central Reserve*' document provided. Specifications for the proposed synthetic oval (Eastern Oval) are understood to be in accordance with the '*Australian Football League and Cricket Australia Handbook of Testing for Synthetic Turf* also provided by the client.

Discussions: The following are requested aspects of the site to be discussed.

- Key issues associated with the sub-surface conditions and how they may impact the proposed infrastructure
- Earthworks
- 1.03 <u>Geology:</u> At least part of the site, as previously indicated, was historically used as a landfill before being converted into parklands and recreational space. The Geological Survey of Victoria, 1:63,360 Series, Ringwood Sheet, indicates the subject site to be underlain by sedimentary deposits which are of the Silurian age and form part of the Dargile Formation. Typically, these deposits comprise of laminated and current bedded sandstones, interbedded with massive siltstones and shales. Some alluvial deposits may exist in the lower elevations.
- **1.04** <u>Field Methods:</u> As part of the geotechnical investigation the following field methods were incorporated:
 - Auger Drilling: All boreholes were drilled using a truck mounted Gemco HP 7 rotary drilling rig equipped with continuous flight 125 millimetre diameter augers fitted with tungsten carbide drill bits.
 - ii) In-situ Vane Shear Strength Testing: In-situ vane shear strength testing was carried out within the cohesive soils at shallow depths using a Pilcon hand vane tester. The tests were conducted in accordance with the test procedure outlined in Australian Standard 1289, "Methods of Testing Soils for Engineering Purposes". Test Method F2. 1, June 1977.
 - iii) Standard Penetration Testing: Standard penetration testing was conducted at regular intervals within the boreholes in accordance with the test procedure outlined in Australian Standard 1289, "Methods of Testing Soils For Engineering Purposes," Test Method 6.3.1, June 1993.in accordance with Australian Standard AS 1726 1993, "Geotechnical Site Investigations."
 - iv) Logging of Soil Profiles: The soil profile encountered in the borehole was logged in accordance with Australian Standard AS 1726 1993, "Geotechnical Site Investigations."

1.05 <u>Laboratory Test Methods:</u> All soil samples were transferred to A.S. James' National Association of Testing Authorities (NATA) registered Clayton South laboratory, where mechanical testing was undertaken by a team of trained laboratory technicians. All laboratory testing was performed in strict accordance with the test methods outlined in Australian Standard AS 1289, "Method of Testing Soils for Engineering Purposes," as follows:

Suggested testing in the brief was modified too.

• Atterberg Limits

AS 1289 Test Method 3.1.1, 3.2.1, 3.3.1.3.4.1

2. RESULTS

2.1 FIELD TESTING

2.1.1 <u>Bore Locations</u>: Twenty (20) boreholes were drilled at the locations indicated on Figures 1 and the logs of these boreholes are given on Figures 2 – 17. The results of standard penetration tests are given on the logs.

(PRELIMINARY) Geotechnical Investigation	24 March 2014
Proposed Eastern Football Hub. Wantirna South Recreation Reserv	e. 50 Mountain Hwy WANTIRNA REF: 115706

2.1.3 <u>Sub-Surface Soil Profile:</u> The results of the borehole drilling program conducted at the site, have been summarised in Table 1 & 2 below.

Borehole	Depth Range of crushed rock (m) (FILL)	Depth Range of Filling (Sandy silt) (m) (FILL)	Depth Range of Silt/ Clay Cap (m) (FILL)	Depth Range of Historic Tip (m) (FILL)	Depth Range of Reworked Natural Material (m) (FILL)	Depth Range of Natural Clayey Silt (m) (SILT) (MH)	Dargile Formation Depth Range of Natural Silty Clay (m) (CLAY) CH /CL
1	-	0-0.2	0.2-0.6	0.6-4.0	-	-	4.0-4.5
2	-	0-0.6	-	0.6-2.0	-	- /	
3	-	0-0.2	-	0.2-3.0	-		
4	-	0-0.1	-	0.1-2.0	-		_
5	0-0.1	-	-	0.1-2.0	-		-
6	0-0.1	-	0.1-0.6	0.6-2.0	-	-	-
7	-	-	0-2.2	2.2-3.0	× · \		-
8	0-0.15	-	-	-	0.15-1.7	-	1.7-3.0
9	0-0.1	-	-	-	0.1-3.0	_	3.0-3.4
10	0-0.05	-	0.05-1.0	1.0-1.4	-	1.4-1.6	1.6-6.4
11	0-0.05		0.05-1.6	1.6-3.5	<u> </u>	-	3.5-6.4
12	0-0.05	-	0.05-1.7	1.7-4.0	y -	-	4.0-6.4
13	0-0.05	-	$\langle \mathcal{N} \rangle$	- - -	0.05-2.5	-	2.5-6.0
14	-	-	1.4-1.8	1.8-4.5	0-1.4	-	4.5-4.6

Table 1: Summary of Sub-surface Soil Profiles within Depths Investigated (Bores 1-14).

Borehole	Depth Range of Filling (Silty Sand) and (Sand / Silt / Gravel) (m) (FILL) Playing Surface	Depth Range of Reworked Natural Material (m) (FILL)	Depth Range of Natural Silt (m) (SILT) (MH)	Dargile Formation Depth Range of Natural Silty Clay or Silty Sand (m) (CLAY) CH /CL, (SAND) (SM)
15	0-0.3	-	-	0.3-2.0
16	0-0.3	0.3-1.0	-	0.3-1.0
17	0-0.3	-	-	0.3-2.0
18	0-0.3	0.3-1.3	-	1.3-2.0
19	0-0.1	-	0.1-0.3	0.3-2.0
20	0-0.3	0.3-1.0	-	0.3-1.0

Table 1: Summary of Sub-surface Soil Profiles within Depths Investigated (Existing Sporting Oval).

Bores 1-4 were carried out at the picnic / play area of the site, bores 5, 6, 7, 8, 9 & 14 were carried out in the car-park area of the site and bores 10-13 were carried out within the proposed new double story building envelope. Bores 15-20 were carried out on the existing sports oval.

A summary of each of the soil units encountered at the subject site is provided as follows:

- <u>Crushed Rock Fill:</u> *Car Park Areas:* Within the car parking areas, a layer of dry, medium dense silty and sandy crushed rock fill of thickness ranging from 0.05-0.15m generally was encountered. The suitability for re-use around the site or as future sub-base has not yet been determined.
- <u>Sandy Silt: *Picnic / Play Area*</u>: A layer of generally brown, dry, medium dense sandy silt with an organic content was encountered within the picnic / play area of the site, ranging in depths of 0.1-0.6 m.
- <u>Sand:</u> 'Capping' Fill used to develop the playing surface of the existing sports oval: A thin layer of moist brown silty sand in medium dense condition was encountered approximately 0.1 m in depth over the existing sports oval to the east. Underlying this a layer of brown sand / silt / gravels was encountered of medium density, 0.2 m thick.
- <u>Silt / Clay Capping</u>: Predominantly within the existing car parking areas and accessible locations of the proposed double story building envelope. A likely reworked natural material was encountered, mostly a yellow brown / orange brown occasionally mottled red silty clay however

appearing as a clayey silt at times. This layer was typically of stiff consistency or medium dense, dry tending moist with a gravel content.

- <u>Historic Landfill:</u> Underlying much of the site in particular the proposed western natural grass oval is the historic tip. This unit typically presented as a grey brown / brown grey moist silty clay of medium density. Accessory materials encountered includes, glass, brick, plastic, metal, gravels and other miscellaneous items such as the sole of a shoe encountered in BH14. Within the depths investigated this unit ranged from 0.1 m up to 4.0 m below ground surface. Typically over the picnic / play area the sandy silt was overlying the historic land fill and within the car park and proposed building envelope where encountered, the historic landfill was underlying the silt / clay capping. Perched water tables were encountered at BH1 and BH14 at approximately 3.0 m above the underlying clay.
- <u>Reworked Natural Material:</u> Encountered at boreholes in close proximity to the existing sporting oval and eastern parts of the sporting oval. Reworked natural materials were encountered of characteristics to those similar to the natural undisturbed materials underlying the site in addition to what appears to be a crushed siltstone fill. This profile generally appeared as a dry to moist yellow brown / orange brown / grey occasionally mottled red stiff silty clay, in medium dense condition.
- <u>Natural Clayey Silt (MH)</u>: Observed in BH10 below the historic tip profile a thin layer of pale grey moist, loose to medium dense clayey silt was encountered. It is possible this was the original ground surface level before filling associated with the historic tip began.
- <u>Natural Silurian Age Dargile Formation:</u> Encountered underlying historic tip fill, natural clayey silts or reworked natural materials, a grey / orange brown stiff silty clay in moist condition was encountered. At deeper depths greater than 5.0 m sub-angular ironstones were encountered including variably ferruginous occasional grit and gravel bands. Of note, the silty clay tended to a sandy / silty clay at approximately 5.0 m within BH11.
- **2.1.5** Discussion of possible historic earthworks: With reference to the attached Cross Section A-A', the following is observed site notes regarding the possible previous earthworks at the site. The bore logs indicate that a possible earthworks program previously conducted at the site included the cutting of natural materials from the eastern areas of the existing oval and subsequent benching by filling of the remaining areas, westwards over the oval. In addition, placement of natural materials over the historic landfill, predominantly over the car park areas and existing

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building area. It appears the materials were also placed in a somewhat uncontrolled manner. The historic landfill appears to reduce in depth as it approaches the existing oval however, relatively significant depths of landfill were still recorded west of the existing pavilion adjacent to the sports oval.

2.1.6 Ground Water: No permanent free ground water was encountered at the time of the site investigation and none would normally be expected within the depths investigated. However at two locations a perched water table was encountered within the historic landfill profile at approximately 3.0 m. It should be noted, however, that following prolonged periods of rainfall the surface fills, historic landfill and reworked natural materials, clayey silt and shallow silty clay may be susceptible to moisture ingress, thereby significantly reducing the workability and strength of both the surface fill and the underlying sands at shallow depths.

2.2 LABORATORY TESTING

- 2.2.1 <u>Test Program:</u> Upon receipt in the laboratory, the disturbed soil samples retrieved from Boreholes were prepared as necessary before Atterberg limit testing was carried out.
- 2.2.2 <u>Test Results:</u> The results of the laboratory Atterberg limits are given attached.

3. DISCUSSION & RECOMMENDATIONS

- 3.1 PROPOSED STRUCTURE: Double storey, well-articulated and able to accommodate minor movement.
- **3.1.1** <u>Site Classification:</u> In selecting an appropriate foundation arrangement the following factors have been taken into account:
 - Sub-surface soil profiles and results of visual site observations
 - Local geological conditions;
 - Knowledge of the site and previous likely earthworks on this development.

If required the site should be classified as Class 'P' in accordance with AS 2870 - 2011 and fully suspended construction will be necessary.

- **3.1.2** <u>Piled Footings:</u> Pile footings should be adopted. Considering the variation of historic landfill depths, the piles need to extend into the silty clay below any clayey silt, reworked natural material or historic fill.
- **3.1.3 Bored Piers:** Bored piers could be considered although some instability may be encountered and temporary liners will be necessary.

If required for a preliminary design a bearing pressure of 300 kPa could be adopted subject to a penetration of 1.0 m into the underlying clay. A skin friction of 30 kPa could be adopted below this level.

3.1.4 Provided that adequate protection is provided against the possibly aggressive ground conditions that may prevail with the historic fill profile, the use of precast concrete piles appears appropriate for the proposed development.

<u>Pile Load Capacity:</u> Provided that adequate protection is provided against the possibly aggressive ground conditions that may prevail within the historic fill profile, the use of precast concrete piles appears appropriate for the proposed development.

Considering the potential for variation of the subsurface profile across the site the piles will need to be driven into the "Dargile Formation" layer and piles need to be driven until the required set is achieved.

The load carrying capacity of driven piles in the stiff clay will significantly be shaft resistance. In this site, it is expected significant load development will not occur until up to in all likelihood 4.0 metres due to the historic landfill and possibly uncontrolled fill. Obviously, variability in both the depth of historic fill and the properties of the Dargile Formation across the site will influence final piling depths and embedment depth variation will exist.

As per our calculations, based on a conservative depth of fill at 4.0m a 350 sq. smooth pile driven 3.0 metres into the Dargile Formation will provide a working load of 160 kN. That is a total pile depth of approximately 7.0m, although site variation and density will determine ultimate depths, and increased pile sizes could be considered. Greater bearing pressures can be provided subject to further geotechnical investigation to deeper depths within the 2-story building envelope until rock is encountered, this is estimated in the order of 8-10 m below ground surface level.

Piling contractors should make their own assessment of piling conditions and load carrying capacities of proprietary pile types. Design geotechnical strength is the calculated design ultimate geotechnical strength multiplied by a geotechnical strength reduction factor. As per the piling code (Revision of AS 2159-1995) the geotechnical strength reduction factor should be determined considering the site, design and installation of piles.

We emphasise that the historic landfill depths are variable up to 4.0 m recorded within the building envelope and variable depths can be expected and will occur.

- **3.1.5** The use of CFA (Continuous Flight Auger) or driven piles could be considered and the logs should be referred to proprietary piling contractors.
- **3.1.6** <u>Pile Settlements:</u> Pile settlements could be assessed using a Young's Moduli (E) for the stiff silty tending sandy clay of 50 MPa.
- **3.1.7** <u>Pile Groups:</u> The total working load for the pile will be developed by a combination of end bearing and shaft friction into the silty tending sandy clay.

It is recommended that the following precautionary measures be taken during the pile driving operations.

- Continuous monitoring of pile heads during installation to ensure eccentricities do not develop.
- A construction sequence be developed to minimise potential interaction (suggest three diameters) of piles and drive central, inner and outer.
- Pre-bore (with casing, if necessary) to remove material and obstructions in the historic landfill.
- Ensure the use of a proprietary tensile capacity joint or during monitoring (both laterally and vertically) have the capacity to 'reset' piles lifting.

It is recommended the driven piles be initially checked using dynamic pile testing (Pile Driving Analyser) and confirmed using CAPWAP analysis or similar on selected piles (10% in Number, but subject to Quality Risk Factors).

3.1.8 <u>Pile Downdrag</u>: The down-drag stress at any point on the pile within the historic landfill could be calculated using the following equation:

	$f_{nf}=0.3\sigma'_{v}$
where f _{nf}	is the downdrag stress
σ'_{v}	is the effective overburden stress (adopt a bulk unit weight
	of 15 kN/m ³ for the fill including capping, reworked natural
	materials and historic landfill)

Pile down drag should be considered to a depth 1.0m above the base of the stiff clay. Based on the information this can be considered at approximately 3.0 m in depth.

- **3.1.9 Protection of Piles:** The possibility remains of the fill to be of aggressive nature to concrete, in all likelihood it will be necessary to provide an appropriate level of protection for the proposed precast concrete piles. Such protection should include the use of cement rich, high quality, very dense, impervious concrete, which should be placed with a high degree of vibration. As a guide the concrete should contain not less than 425 kg/m3 cement and 50 kg/m3 flyash placed with a super-plasticiser at a water cement ratio of 0.30. Care will also need to be taken to ensure that the steel segmented pile joints are not located within the uppermost 4.0 m. Other proprietary options could also be considered.
- 3.1.10 <u>Earthquake Loading</u>: In accordance with Australian Standard 1170.4-2007, Part 4, "Earthquake Actions in Australia", site sub-soil class of C_e shallow soil site and Hazard Factor (Z) of 0.09 should be adopted for the design of the proposed structure at the subject site.
- 3.1.11 Discussion of Settlements of proposed premier quality natural turf, sand based oval (Proposed Western Oval): In accordance with the provided likely specifications for the proposed premier quality natural turf sand based oval taken from the Colac Central Reserve, it is stated within Section 8.1 of the document that the 'soil rootzone layer' will be to a consolidated depth of 250 mm so that the finished surface has a tolerance of + or 10 mm.'

Given the ongoing settlement that will occur underlying the site and the relatively poor engineering quality of the landfill materials, it is envisioned that due to these inevitable settlements / differential settlements that will take place at the proposed location, the tolerance of + or - 10mm will be significantly exceeded. Drainage in particular is likely to be affected due to the differential settlement of the oval.

Significant cost will be expended in remediating this area of the site and in fact would likely be cost prohibitive. A 'use and maintain' approach would appear all that is reasonable.

3.1.12 Discussion of Settlements of Australian Football League and Cricket Australia Grade Synthetic Turf Oval (Eastern Oval): After reviewing the provided handbook, regarding the specifications for a synthetic oval. Settlement tolerances were not provided, however it is assumed that this type of construction, similar to the natural turf, sand based oval is sensitive to movements and drainage is critical.

As the synthetic turf oval is proposed to be located at the existing sports oval. Given the presence of reworked natural fill materials, particularly from the centre of the existing sporting oval heading westwards. The likely resulting consolidation settlements are expected to be outside + or -10 mm, if the proposed synthetic turf oval was to be located at the existing ovals location. The use of an impact roller on this existing fill may be sufficient to reduce the settlements in the fill to acceptable limits. However, more and intrusive investigation is required with test pits.

3.1.13 <u>Discussion of Environmental Condition of Site:</u> Given the site history as to a landfill. It is understood there are no available records to the type and quantity of material that was bought on site. Due to the uncontrolled nature of the filling, it is possible and likely that the soil is contaminated to some degree, however to what degree at this stage is largely unknown and until firm proposals exist no testing is warranted.

Landfills can be typically heterogeneous in both physical and chemical composition. As such, environmental preliminary sampling and analytical testing can be misleading as both the physical and chemical composition can change over relatively short distances. Therefore, more detailed environmental investigations are recommended as follows; however, this is outside the scope of this investigation.

A preliminary phase 1 environmental assessment focuses on site history in the aim of determining possible environmental impacts by ascertaining possible contaminants of concern related to the sites previous use.

Upon the completion of a site history study, a limited sampling and analysis program can be completed testing for contaminants of concern using a specific sampling strategy. Collection of ecological information on background inorganic element and organic compounds present would also be completed. In doing so, this gathered information would aid in reviewing results and determining if possible impacts may be imposed on ecological receptors. Contaminant threshold concentration levels can be adopted for human health and ecological receptors in accordance with the current '*National Environmental Protection (Assessment of Site Contamination) Amendment Measure 2013 (No.1)*.'

A preliminary off-site disposal categorisation can be provided in accordance with Victorian EPA Publication IWRG-621-June 2009 'Soil Hazard Categorisation and Management'. Along with any necessary details and recommended practices for disposing the soil off-site. It should be noted as above, given the likely heterogeneous nature of landfill material a conservative approach is likely to be adopted.

The production of landfill gases will also require investigation at the social and change room complex.

Note: Given the high content of municipal/domestic waste and industrial wastes in the historic landfill profile, the fill cannot be used as engineered / structural fill. A prescribed industrial waste category will need to be assigned to the landfill material and if required disposed of at a licenced facility.

Upon the completion of the preliminary Phase 1 environmental assessment, a further phase 2 investigation may be recommended to further explore / delineate any contamination found or determine an appropriate remediation strategy in line with a hierarchy of control.

In the case construction was to commence at the site, a site specific environmental management plan should be completed addressing any contamination on site to inform any contractors of the conditions, and how to manage them for both safety, and compliance.

3.2 PAVEMENT CONSTRUCTION & GROUND FLOOR SLAB

3.2.1 <u>Pavements:</u> All building pavements should be fully suspended.

3.2.2 External Pavements Constructed on Fill: Some ongoing settlements within the fill underlying the subject site will result in distortion and maintenance requirements to pavements constructed on the existing ground surface level. Recognising this, any proposed pavements should be constructed on an adequately prepared subgrade with maximum possible grades to minimise future maintenance and ponding of water. Settlements are difficult to assess however may be as much as 50mm. (Except for the landfill area)

Flexible pavements and floor slabs constructed on an adequately prepared fill subgrade may be designed using a Design CBR value of 2.0 % Rigid pavements, on the other hand, constructed on a similar subgrade, may be designed using a Modulus of Subgrade reaction value of 20 kPa/mm.

Alternatively, rigid pavements may be designed in accordance with the Cement and Concrete Association of Australia, 1997 publication, "Industrial Pavements - Guidelines for Design, Construction and Specification" using long and short term Young's Moduli of 10 and 12 MPa respectively.

The effects of movements on any proposed rigid pavements can be minimised by incorporation of positive load transfer devices such as dowels.

It should be pointed out, however, that the pavement design parameters recommended above are given subject to the subgrade preparation outlined in Clause 3.2.2 and 3.2.3 being carried out, in addition to adequate subgrade drainage control, as outlined in Clause 3.2.4.

3.2.3 Preparation Fill Subgrade: The fill subgrade should be compacted with a heavy weight vibrating sheepsfoot roller. A minimum dry density ratio of 98% of the maximum dry density value determined by the Standard Compaction Test in accordance with Australian Standard AS1289 5.1.1 - 1993 should be achieved.

Under extreme conditions, if the subgrade exists in a saturated state, it may be necessary to strip the saturated clays and replace these with suitable granular fill compacted to a dry density not less than 98% of the maximum dry density value determined by the Standard compaction test in accordance with AS1289 5.1.1.

It should be appreciated that the long term performance of the proposed pavements constructed on a fill subgrade significantly depends on the subgrade moisture conditions at the time of construction.

If very wet conditions are prevalent at the time of construction then there is the risk of some subsequent shrinkage occurring as the clay dries out. On the other extreme, if very dry conditions have prevailed for a significant period then there could be a risk of some resulting heave as the clays wet up.

As outlined above, impact rolling could be considered to improve performance.

- **3.2.4** <u>Long Term Subgrade Moisture Control:</u> It is considered essential for the long term performance of the proposed pavements at the subject site that both an effective surface and lateral cut-off drainage system be provided and maintained to minimise the risk of moisture migration into both the pavement sub-base and subgrade layers. Under no circumstances should the pavement and subgrade layers be permitted to remain in a saturated condition.
- **3.2.5** <u>Earthworks:</u> It is pointed out that the clay/silt fills including the capping layer over much of the site are notoriously difficult to work and if not compacted at or very close to the optimum moisture content, can exhibit measurable volume change with time. As such, the use of these clays will require close supervision.

Any imported structural fill should essentially be of a granular nature. All fill material should have a nominal particle size of 75 millimetres or less and, if required, a guide for selecting an appropriate material would be as follows:

• Plasticity Index. x Percentage Passing 0.425 millimetres (AS Sieve) less than or equal to 600

Structural fill should be compacted in layers not greater than 200 millimetres when loose and should be compacted to a dry density not less than 98% of the maximum dry density value determined by the Standard Compaction Test in accordance with Australian Standard AS 1289 5.1.1 - 1993 using an appropriate heavy weight vibrating roller.

During compaction, the fill material should have moisture content within the range 85% to 115% of the optimum moisture content as determined by the Standard Compaction Test in accordance with AS 1289 E1.1 - 1993.

3.2.6 <u>Fully Suspended Construction:</u> If the anticipated pavement settlements cannot be tolerated it will be necessary to fully suspend the proposed pavements on a series of piles.

3.3 GENERAL

- **3.3.1** <u>Underground Services:</u> Ongoing settlements within the significant depths of fill underlying the subject site will result in distortion and maintenance requirements to underground services at the subject site. Recognising this, the following precautions should be taken for any proposed underground services at the subject site:
 - All services should be laid with maximum possible grades to minimise future maintenance requirements.
 - Flexible joints should be incorporated for all services.
 - Services connecting to the proposed structure should be appropriately sleeved at the point of connection to allow for differential settlements where the structure is suspended.
- **3.3.2** <u>Inspection of Footing Excavations:</u> All footing excavations must be examined to ensure that the required founding soil has been exposed. Any unusual features must be reported to this office immediately in order to ensure that the recommendations outlined in this report remain relevant.
- **3.3.3** <u>General:</u> The Modulus of Subgrade reactions specified throughout the report are referred to as the K(0.3) value in most literature on the subject. As such, they are directly relevant where point loads are critical, but otherwise will require amendment depending on the value of the loading and geometry of the structural element involved. We would be pleased to advise further once relevant details are available.

Conditions may change with the seasons. In particular, the surface silts and residual clays underlying the site at shallow depths may become saturated and unworkable following prolonged periods of rainfall.

The above recommendations are based on the bore and test results, together with experience of similar conditions and are expected to be typical of the area or areas being considered. Nevertheless, all excavations should be examined carefully and any unusual feature reported to us in order to determine whether any changes might be advisable.

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REVIEWED BY T.J. HOLT MIEAust CPEng EC-1022 A.S. JAMES PTY LTD J .T. HOLT BEng Grad Inst of Engineers

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A	A.S.JAMES PTY. LTD.	Location: PROPOSED EASTERN Borehole: 1					orehole: 1
G	eotechnical Engineers		FOO	OTBALL	HUB, WAN	TIRNA	Data Marita
		Job No. Ground W	11570 ator:	J6 Porcho	d water table	at annr	Date: Mar '14
Soil Type	Description	Depth		Tests	Results	αι αρρι	oximately 5.0 m.
<u> </u>							
FILL	Brown Silt	0					
	Accessory Materials: Trace sand and grass roots	0.2					
FILL	Yellow brown / orange brown stiff clay	0.2 .	///				
	Accessory Materials: Gravels, silt		///				
	Dry, medium dense						
	Likely Reworked Natural Material	0.6.					
			///				
			///				
			///				
FILL	Brown grey / grey brown firm clay		///				
	Accessory Materials: Silt, plastic, brick fragments,						
	glass, gravels Moist modium donso						
	Historic Landfill		$\langle / / \rangle$				
			///				
			///				
		•					
			///				
			///				
		•					
			$\langle / / \rangle$				
			///				
CLAY	Grey / orange brown	4.0	///				
(CH/CL)	Silty Moist stiff						
	Dargile Formation						
	BOREHOLE TERMINATED	4.5					
		· ·					
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohes	ion	L.L. Liquid Li	imit	
I Undistur	rbed Sample - Diameter Stated	Ø Friction	Angle		P.L. Plastic I	Limit	Figure
s Vane S	hear Strength	P Wet De	nsity		P.I. Plasticit	y Index	2
p Pocket	Penetrometer Resistance	w Moisture	- Conter	nt	L.S. Linear S	Shrinkaqe	

A	A.S.JAMES PTY. LTD.	Location: PROPOSED EASTERN Borehole: 2					
G	eotechnical Engineers	Job No 11	FOOTBALI	L HUB, WANTIRNA	Date: Mar '14		
		Ground Wate	er: NIL		Dute: Mar 14		
Soil Type	Description	Depth					
FILL	Grey brown silt Accessory Materials: Trace gravels Dry, medium dense	0 0.6 .					
FILL	Brown grey / yellow brown / grey firm clay Accessory Materials: Silt, plastic, gravels Dry to moist, medium dense Historic Landfill						
	BOREHOLE TERMINATED Note: This bore was terminated still within the historic landfill profile	2.0					
. Otrad	d Department Test. N blows (170 mm in an						
+ Standar I Undistu s Vane S p Pocket	a Penetration Test - N blows/150mm. incr. rbed Sample - Diameter Stated hear Strength Penetrometer Resistance	c Apparent Co Ø Friction Ang P Wet Density w Moisture Co	onesion gle y ontent	L.L. Liquid Limit P.L. Plastic Limit P.I. Plasticity Index L.S. Linear Shrinkage	Figure 3		

A	A.S.JAMES PTY. LTD.	Location: PROPOSED EASTERN Borehole: 3					
G	eotechnical Engineers	1	FOOTBAL	L HUB, WANTIRNA	_		
		Job No.	115706		Date: Mar '14		
Soil Type	Description	Depth	Tests	Results			
FILL	Grey brown silt Accessory Materials: Gravels	0	777				
	Dry, medium dense						
FILL	Brown grey / orange brown / grey firm clay Accessory Materials: Silt, plastic, gravels, copper Dry to moist, medium dense Historic Landfill Note: Very Moist at the bottom of the borehole						
	BOREHOLE TERMINATED	3.0					
+ Standar I Undistur s Vane S p Pocket	d Penetration Test - N blows/150mm. incr. rbed Sample - Diameter Stated hear Strength Penetrometer Resistance	c Apparen Ø Friction P Wet Der w Moisture	t Cohesion Angle nsity e Content	L.L. Liquid Limit P.L. Plastic Limit P.I. Plasticity Index L.S. Linear Shrinkage	Figure 4		

A	A.S.JAMES PTY. LTD.	Location:	PROPOSEI	DEASTERN B	orehole: 4 & 5
G	eotechnical Engineers		FOOTBALL	HUB, WANTIRNA	
		Job No.	115706		Date: Mar '14
Soil Type	Description	Depth	Tests	Results	
FILL	Grey brown silt Accessory Materials: Gravels, grass roots Dry, medium dense	0 0.1 .			
FILL	Grey brown / orange brown / grey firm clay Accessory Materials: Silt, plastic, gravels, glass Dry to moist tending very moist, medium dense Historic Landfill				
BH5	BOREHOLE TERMINATED	2.0			
FILL	Accessory Materials: Silt, sand Dry, medium dense	0.1.			
FILL	Grey brown / orange brown / grey stiff clay Accessory Materials: Silt, cloth, gravels, steel Dry to moist, medium dense Historic Landfill				
	BOREHOLE TERMINATED	2.0			
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparent	Cohesion	L.L. Liquid Limit	
I Undistu	rbed Sample - Diameter Stated	Ø Friction	Angle	P.L. Plastic Limit	Figure
s Vane S	hear Strength	P Wet Den	sity	P.I. Plasticity Index	5
p Pocket	Peneirometer Resistance	w Moisture	Content	I S Linear Shrinkade	1

4	Location	PROP	OSED	EASTERN	Borehole: 6	
G	eotechnical Engineers		FOOT	BALL I	HUB, WANTIRN	A
		Job No.	115706 ator: NII			Date: Mar '14
Soil Type	Description	Depth		ests	Results	
FILL	Grey brown crushed rock Accessory Materials: Silt, sand	0 0.1 .	777			
FILL	Dry, medium dense Yellow brown / orange brown mottled red stiff clay					
	Accessory Materials: Gravels, silt Dry, medium dense Likely Reworked Natural Material (Capping Laver)	 0.6 .				
FILL	Grey tending dark grey firm clay					
	Accessory Materials: Silt, plastic, glass, steel Moist, medium dense Historic Landfill	· · · ·				
	BOREHOLE TERMINATED	2.0				
						1
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohesion		L.L. Liquid Limit	Figure
I Undistu	rbed Sample - Diameter Stated	Ø Friction	Angle		P.L. Plastic Limit	
p Pocket	Penetrometer Resistance	w Moisture	e Content		L.S. Linear Shrinka	ge

A.S.JAMES PTY. LTD.		Location: PROPOSED EASTERN Borehole: 7					
Geotechnical Engineers			FOOTBAL	L HUB, WANTIRNA			
		Job No.	115706		Date: Mar '14		
Soil Type	Description	Ground W	ater: NIL	Besults			
FILL	Yellow brown silt	0	7.7.7	riesuits			
	Accessory Materials: Silt, sand, tending clayey Dry to moist, medium dense Likely Reworked Natural Material (Capping Layer)	· · ·					
		· · · · ·					
		 2.2 .					
FILL	Dark grey brown firm clay Accessory Materials: Silty, plastic, glass, steel, brick fragments Moist, medium dense Historic Landfill						
	BOREHOLE TERMINATED	3.0	///				
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparent	Cohesion	L.L. Liquid Limit	Figure		
I Undistui	bear Strength	P Wat Day	Angle	P.L. Plastic Limit	rigure 7		
s vane Snear Strengtn p Pocket Penetrometer Resistance		w Moisture	Content	L.S. Linear Shrinkage			

A.S.JAMES PTY. LTD.		Location: PROPOSED EASTERN Borehole: 8					
Geotechnical Engineers			FO	OTBALI	HUB, WANTIRNA		
		Job No.	11570	06		Date: Mar '14	
Soil Type	Description	Depth	aler.	Tests	Results		
FILL	Grey brown crushed rock Accessory Materials: Silt, sand	0	(//)				
FILL	Dry, medium dense Yellow brown / orange brown mottled red stiff clay / silt Accessory Materials: Gravels Dry to moist, medium dense Likely Reworked Natural Material	0.15 .					
FILL / CLAY (CH/CL)	Orange brown stiff clay stiff clay Accessory Materials: Silty Dry to moist, medium dense Possibly Reworked Natural Material	1.3 . 1.7 .					
CLAY (CH/CL)	Grey / orange brown Silty Moist, stiff Ironstone (Sub-angular) gravels present at deeper depths including occasional grit and gravel bands, variably ferruginous Dargile Formation BOREHOLE TERMINATED						
+ Standard Penetration Test - N blows/150mm. incr.		c Apparent Cohesion		ion	L.L. Liquid Limit P.L. Plastic Limit	Figure	
s Vane Shear Strength		P Wet Density			P.I. Plasticity Index	8	
p Pocket Penetrometer Resistance		w Moisture Content			L.S. Linear Shrinkag	e	

A.S.JAMES PTY. LTD.		Location: PROPOSED EASTERN Borehole: 9					
Geotechnical Engineers		loh No	FOOTBAL	L HUB, WANTIRNA	Data: Mar 114		
		Ground W	ater: NIL		Date: Mar 14		
Soil Type	Description	Depth	Tests	Results			
FILL	Grey brown crushed rock Accessory Materials: Silt, sand Dry, medium dense Yellow brown / orange brown / grey mottled red stiff clay / silt Accessory Materials: Gravels Dry to moist, medium dense Note: tending to clay	0 0.1 .					
	Likely Reworked Natural Material						
FILL	Accessory Materials: Silty Dry to moist, medium dense Likely Reworked Natural Material	1.0 .					
FILL	Dark brown / orange firm clay Accessory Materials: Silty Moist, medium dense, Note: Auger grabbing Possibly Reworked Natural Material	2.3 . 2.6 .					
FILL / CLAY (CH/CL)	Dark brown / orange / grey stiff clay Accessory Materials: Silty Moist, medium dense Possibly Reworked Natural Material	3.0	// +	N = 3/5/6			
CLAY (CH/CL)	Grey / orange brown Silty Moist, stiff Dargile Formation	3.4 .					
	BOREHOLE TERMINATED						
		· · · · · · · · · · · · · · · · · · ·					
+ Standard	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohesion	L.L. Liquid Limit			
I Undistur	bed Sample - Diameter Stated	Ø Friction	Angle	P.L. Plastic Limit	Figure		
s Vane Shear Strength p Pocket Penetrometer Resistance		P Wet Der w Moisture	nsity e Content	P.I. Plasticity Index L.S. Linear Shrinkage	9		

A.S.JAMES PTY. LTD.		Location: PROPOSED EASTERN Borehole: 10				
Geotechnical Engineers		1	FOC	TBAL	L HUB, WANTIRNA	B
		Job No.	11570)6 NIII		Date: Mar '14
Soil Type	Description	Depth	aler.	Tests	Results	
FILL	Grey brown crushed rock Accessory Materials: Silt, sand	0 0.05 .				
FILL	Dry, medium dense Orange brown / grey stiff clay Accessory Materials: Gravels, silty Dry to moist, medium dense, Likely Reworked	1.0				
FILL	Natural Material (Capping Layer) (0.05-1.0 m) Brown gravels / silt Accessory Materials: Clay, plastic, brick fragments, glass, steel Dry to moist, medium dense Historic Landfill (1.0-1.4 m)	1.4 . 1.6 .		+	N = 3 / 3 / 4	
SILT (MH)	Pale grey Clayey Maist loose to medium dones (1.4.1.6 m)	2.5 .				
CLAY (CH/CL)	Dark Brown mottled orange /grey Silty Moist, stiff (1.6-2.5 m)	· · · · · · · · · · · · · · · · · · ·		+	N = 2 / 3 / 4	
CLAY (CH/CL)	Grey / orange brown Silty Moist, stiff Ironstones (Sub-angular) present and deeper depths including occasional grit and gravel bands, variably ferruginous Dargile Formation			÷	N = 4 / 6 / 7	
	BOREHOLE TERMINATED	 6.4 .		+	N = 5 / 9 / 10	
 + Standard Penetration Test - N blows/150mm. incr. I Undisturbed Sample - Diameter Stated s Vane Shear Strength 		c Apparent CohesionØ Friction AngleP Wet Density		L.L. Liquid Limit P.L. Plastic Limit P.I. Plasticity Index	Figure 10	
p Pocket Penetrometer Resistance		w Moisture Content			L.S. Linear Shrinkage	
A.S.JAMES PTY. LTD.		Location:	PRC	OPOSE	DEASTERN E	Borehole: 11
---------------------	--	---------------------------------------	-----------	--------	-----------------------	---------------
G	eotechnical Engineers		FOC	OTBALI	L HUB, WANTIRNA	
		Job No.	11570	06		Date: Mar '14
Soil Type	Description	Depth	ater:	Tests	Results	
FILL	Grey brown crushed rock	0		10313	ricouito	
	Accessory Materials: Silt, sand Dry, medium dense (0-0.05 m)	0.05 . 0.3 .	\square			
FILL	Accessory Materials: Silty Dry, medium dense, (0.05-0.3 m)	· ·				
FILL	Orange brown / grey stiff clay Accessory Materials: Gravels, silty Dry to moist, medium dense, Likely Reworked	1.6 .				
	Natural Material (Capping Layer) (0.3-1.6 m)	•				
FILL	Grey brown / orange brown / dark grey firm clay Accessory Materials: Silt, plastic, gravels, glass Moist, loose to medium dense Historic Landfill (1.6-3.5 m)					
	Note: Silty Clay lenses	· · · · · · · · · · · · · · · · · · ·				
CLAY (CH/CL)	Orange brown /grey Silty Moist, stiff	3.5 .				
CLAY (CH/CL)	Grey / orange brown Silty, tending sandy Moist, stiff Ironstone (Sub-angular) gravels present at deeper depths including occasional grit and gravel	5.0 5.0		+	N = 4 / 6 / 7	
	bands, variably ferruginous Dargile Formation BOREHOLE TERMINATED	 6.4 .		+	N = 6 / 7 / 9	
		· · ·				
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohes	ion	L.L. Liquid Limit	
I Undistu	rbed Sample - Diameter Stated	Ø Friction	Angle		P.L. Plastic Limit	Figure
s Vane S	hear Strength	P Wet Der	nsity		P.I. Plasticity Index	11
p Pocket	Penetrometer Resistance	w Moisture	e Conter	nt	L.S. Linear Shrinkage	

A.S.JAMES PTY. LTD.		Location:	PROPOS	ED EASTERN	Borehole: 12
G	eotechnical Engineers		FOOTBAL	L HUB, WANTIRNA	B
		Job No. Ground W	115706 ator: NII		Date: Mar 14
Soil Type	Description	Depth	Tests	Results	
FILL	Grey brown crushed rock Accessory Materials: Silt, sand Dry, medium dense (0-0.05 m)	0 0.05 .			
FILL	Orange brown / grey stiff clay Accessory Materials: Gravels, silty Dry to moist, medium dense, Likely Reworked Natural Material (Capping Layer)	· · · · · · · · · · · · · · · · · · ·			
FILL	Dark Grey brown firm clay Accessory Materials: Silt, plastic, gravels, sand Moist, loose to medium dense Historic Landfill				
CLAY (CH/CL)	Grey / orange brown Silty Moist, stiff Ironstone (Sub-angular) gravels present at deeper depths including occasional grit and gravel bands, variably ferruginous Dargile Formation	4.0	+	N = 4 / 6 / 8 N = 11 / 13 / 14	
	BOREHOLE TERMINATED				
+ Standar I Undistur s Vane S p Pocket	d Penetration Test - N blows/150mm. incr. rbed Sample - Diameter Stated hear Strength Penetrometer Resistance	c Apparent Ø Friction P Wet Der w Moisture	Cohesion Angle hsity Content	L.L. Liquid Limit P.L. Plastic Limit P.I. Plasticity Index L.S. Linear Shrinkage	Figure 12

A.S.JAMES PTY. LTD.		Location:	PROPOSE	ED EASTERN	Borehole: 13
G	eotechnical Engineers		FOOTBAL	L HUB, WANTIRNA	
		Job No.	115706	[Date: Mar '14
Soil Type	Description	Depth	Tests	Results	
FILL	Grey brown crushed rock Accessory Materials: Silt, sand Dry, medium dense (0-0.05 m)	0 0.05 .	17		
FILL	Orange brown / red / grey stiff clay Accessory Materials: Trace gravels, silty Dry to moist, medium dense, Likely Reworked Natural Material	 1.6 .			
FILL / CLAY (CL / CH)	Orange brown /grey Silty Moist, stiff Possibly Reworked Natural Material	2.5 .	+	N = 7 / 9 / 12	
CLAY (CH/CL)	Grey / orange brown Silty, tending sandy Moist, stiff Ironstone (Sub-angular) gravels present at deeper depths including occasional grit and gravel bands, variably ferruginous Dargile Formation	· · · · · · · · · · · · · · · · · · ·	+	N = 7 / 9 / 10 N = 9 / 12 / 22	
	BOREHOLE TERMINATED	6.4 .			
+ Standard I Undistur s Vane Si p Pocket	d Penetration Test - N blows/150mm. incr. bed Sample - Diameter Stated hear Strength Penetrometer Resistance	c Apparent Ø Friction A P Wet Den w Moisture	Cohesion Angle sity Content	L.L. Liquid Limit P.L. Plastic Limit P.I. Plasticity Index L.S. Linear Shrinkage	Figure 13

4	Location:	Location: PROPOSED EASTERN Borehole: 14					
G	eotechnical Engineers	FOOTBALL HUB, WANTIRNA					
		Job No.	11570	06	d and the state	Date: Mar '14	
Soil Type	Description	Ground Wa	ater:	Percne	d water table at app	roximately 3.0 m.	
- Зоп туре FILI	Orange brown silt	0		16212	nesuits		
	Accessory Materials: Gravels , clayey		777				
	Dry to moist, medium dense		///				
	Likely Reworked Natural Material						
	Note. Tending to sity day		///				
			///				
		•					
			$\langle / / \rangle$				
			///				
			//				
		•					
FILL	Orange brown / grey stiff clay	1.4 .	///				
	Accessory Materials: Gravels , silty		$\overline{//}$				
	Dry to moist, medium dense						
	Capping Laver)	1.8 .	$\langle / / \rangle$				
			///				
			//				
FILL	Grey brown firm clay Accessory Materials: Silt plastic glass steel	•					
	sand, sole of shoe		V//				
	Moist, loose to medium dense		//				
	Historic Landfill						
			V//				
			///				
		•					
			///				
			///				
			$\langle / / \rangle$				
		•	$\langle / / \rangle$				
			///				
			//				
		•					
		4.5	V//				
CLAY	Orange /grey	4.6 .					
(CH / CL)	Silty						
	Dargile Formation						
	BOREHOLE TERMINATED						
		· ·					
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohesi	ion	L.L. Liquid Limit		
I Undistur	rbed Sample - Diameter Stated	Ø Friction	Angle		P.L. Plastic Limit	Figure	
s Vane S	hear Strength	P Wet De	nsity		P.I. Plasticity Index	14	
р Роскет	reneuonneler Resistance	w ivioisture	+ Conter	n.	L.S. Linear Shrinkage	5	

A.S.JAMES PTY. LTD.		Location:	PROPOSE	ED EASTERN B	orehole: 15 & 16
G	eotechnical Engineers		FOOTBAL	L HUB, WANTIRNA	
		Job No.	115706		Date: Mar '14
Soil Type	Description	Depth		1	
BH15 FILL	Grass Playing Surface Brown Sand Accessory Materials: silt, organics	0 0.1 .	77		
FILL	Moist, Medium Dense (0 - 0.1 m) Brown Sand / Silt / Gravels Moist, Medium Dense (0.1 - 0.3 m)	0.3 .		s > 140 kPa	
CLAY (CH / CL)	Orange / grey / red Silty Moist, very stiff				
	Dargile Formation			s > 140 kPa s = 120 kPa	
	BOREHOLE TERMINATED	2.0			
BH16 FILL	Grass Playing Surface Brown Sand Accessory Materials: silt, organics Moist, Medium Dense (0 - 0.1 m)	0 0.1 .			
FILL	Brown Sand / Silt / Gravels Moist, Medium Dense (0.1 - 0.3 m) Orange brown / yellow brown silt Accessory Materials: Clay Dry to moist, medium dense, Likely Reworked Natural Material	0.3 .			
		1.0		s = 114 kPa	
(CH / CL)	Silty Moist, very stiff Dargile Formation			s = 136 kPa	
	BOREHOLE TERMINATED	2.0			
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparent	Cohesion	L.L. Liquid Limit	
I Undistu	rbed Sample - Diameter Stated	Ø Friction A	ngle	P.L. Plastic Limit	Figure
s Vane S p Pocket	hear Strength Penetrometer Resistance	P Wet Dens w Moisture	sity Content	P.I. Plasticity Index L.S. Linear Shrinkage	15

A.S.JAMES PTY. LTD.		Location:	PRO	POSE	D EASTERN	orehole: 17 & 18
G	eotechnical Engineers		FOO	TBAL	L HUB, WANTIRNA	
		Job No.	11570	6		Date: Mar '14
	Description	Ground W	ater: N		Deculto	
Soli Type	Description Grass Playing Surface	Depth		lests	Results	
FILL	Brown Sand	0				
	Accessory Materials: silt, organics	0.1 .	777			
	Moist, Medium Dense (0 - 0.1 m)		///			
FILL	Brown Sand / Silt / Gravels	0.3 .	$\langle / / \rangle$			
	Moist, Medium Dense (0.1 - 0.3 m)					
				•	s > 140 kPa	
01.437						
	Orange /grey					
	Moist verv stiff					
	Dargile Formation				s > 140 kPa	
	5					
		1.3 .				
SAND	Fine to Medoum Grained		11			
(SM)	Grey Brown / yellow brown mottled red		111	•	s > 140 kPa	
	Silty		111			
	Noisi		111			
			///			
	BOREHOLE TERMINATED	2.0	/// ////			
BH18	Grass Playing Surface					
FILL	Brown Sand	0				
	Accessory Materials: silt, organics	0.1 .				
	Moist, Medium Dense (0 - 0.1 m)					
FILL	Brown Sand / Silt / Gravels	0.3 .				
	Moist, Medium Dense (0.1 - 0.3 m)					
FILL	Orange brown / yellow brown silt					
	Accessory Materials: Clay Dry to moist medium dense Likely Reworked					
	Natural Material		$\sqrt{/}$			
			V/V			
			V//			
			V/A			
o		1.3 .	K//			
	Orange / grey brown mottled red				a 102 kBa	
	Moist stiff			•	5 = 102 KFa	
(0117 02)	Note: Possibly reworked Natural Materials					
	BOREHOLE TERMINATED	2.0	μщ			
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohesio	on	L.L. Liquid Limit	
I Undistu	rbed Sample - Diameter Stated	Ø Friction	Angle		P.L. Plastic Limit	Figure
s Vane S	hear Strength	P Wet Der	nsity		P.I. Plasticity Index	16
p Pocket Penetrometer Resistance			e Content	t	L.S. Linear Shrinkage	

A	A.S.JAMES PTY. LTD.		Location: PROPOSED EASTERN Borehole: 19 & 20				
G	eotechnical Engineers		FOC	TBAL	L HUB, WANTIRNA		
		Job No.	11570)6		Date: Mar '14	
Soil Turne	Description	Ground W	ater:		Deculto		
BH19	Grass Plaving Surface	Depth		Tests	nesuits		
FILL	Brown Sand	0					
	Accessory Materials: silt, organics	0.1 .	///				
o	Moist, Medium Dense (0 - 0.1 m)		1				
SILT	Grey brown containing trace gravels and sand	0.3.					
(IVIL)	Moist, Medium Dense (0.1 - 0.3 m)				s > 140 kPa		
CLAY	Grey Brown / orange brown / red at deeper depths						
(CH / CL)	Silty	•					
	Moist, very stiff				s – 130 kPa		
	Dargher offiation			•	5 – 100 Ki a		
					- 110 HD-		
				•	S = 140 KPa		
	BOREHOLE TERMINATED	2.0					
BH20	Grass Playing Surface						
FILL	Brown Sand	0					
	Accessory Materials: silt, organics	0.1 .	\square				
EUI	Moist, Medium Dense (0 - 0.1 m) Brown Sand / Silt / Gravels	03					
	Moist, Medium Dense (0.1 - 0.3 m)	0.0 .	$\forall \forall \forall$				
FILL	Orange brown / grey brown stiff clay		V//				
	Accessory Materials: Silt		V/Λ				
	Dry to moist, medium dense, Likely Reworked		///				
	Natural Material	•					
		1.0	V/A		s = 94 kPa		
			ΉΤ				
CLAY	Grey Brown / orange brown						
(CH / CL)	Silty Moist you stiff	•			c – 90 kPc		
	Dargile Formation			•	5 = 50 KF a		
		20					
		2.0	┝┷┶┷┫				
, Otar-I	d Department Tool N blaue (150mm in m		+ Cah'	00	فاستناعه الماديم (
+ Standar	a Perietration Test - N blows/150mm. Incr.	c Apparen		υΠ		Figure	
	rbed Sample - Diameter Stated	D Hriction	Angle		P.L. Plastic Limit	rigure	
s vane S	near Suengin	P vvet Der	ISILY	+	F.I. Plasticity index		
р Роскет	reneuronneler Resistance	W IVIOISTURE	e Conten	ι	L.S. LINear Shrinkage		



Knox Hockey Facility Feasibility Study

16/12/2019

Appendix D - Environmental Site Assessment





Wantirna Reserve Former Landfill Investigation

DRAFT

16-Aug-2019

Preliminary Environmental Site Assessment

Wantirna Reserve Former Landfill



Wantirna Reserve Former Landfill Investigation Preliminary Environmental Site Assessment

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Preliminary Environmental Site Assessment

Wantirna Reserve Former Landfill

Client: Knox City Council

ABN: 24 477 480 661

Prepared by

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16-Aug-2019

Job No.: 60548185

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Wantirna Reserve Former Landfill Investigation Preliminary Environmental Site Assessment

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Wantima Reserve Former Landfill Investigation Preliminary Environmental Site Assessment

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Quality Information

Document	Preliminary Environmental Site Assessment
Ref	60548185
Date	16-Aug-2019
Prepared by	Breana McCartney
Reviewed by	Vera Levina/ Mark Davidson

Revision History

Rev	Rev Revision Date Details		Authorised			
			Name/Position	Signature		
1	16-Aug-2019	Draft	Vera Levina Principal Hydrogeologist			

Wantima Reserve Former Landfill Investigation Preliminary Environmental Site Assessment

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1.0 Introduction

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AECOM Services Pty Ltd (AECOM) was engaged by Knox City Council (KCC) to undertake a Preliminary Phase 1 Environmental Site Assessment (ESA) desktop study for the landfilled area at Wantirna Reserve (61 Mountain Hwy, Wantirna, Victoria)(Refer to **Figure F1, Appendix A**). The reserve is located at the intersection of Mountain Highway and Eastlink Toll Road (M3). The Wantirna Reserve (the Reserve) is over 10 hectares in size

1.1 Project Background

KCC has recently identified the reserve to be a former landfill, with a 2014 geotechnical investigation by A.S. James Geotechnical (A.S. James, 2014) encountering waste. The exact size, extent and thickness of the landfilled area is currently uncertain. Melways Maps from 1970 and 1971-1972 refer to a 'City of Knox Tip' in two different locations at the site, however KCC records do not confirm this. The site boundary and facilities layout are presented in **Figure 1** (**Appendix A**).

Wantirna Reserve has been used as grounds for various recreational activities and associated facilities (including football oval, cricket, tennis, scouts, walking and cycling path). An investigation to confirm the location and extent of the historical tip in the 1971-1972 Melways Map is required to inform potential infrastructure upgrades at the site. It is also required to confirm whether historical filling was undertaken in the area of the 'City of Knox Tip' outlined in the 1970 Melways map, including the western portion of Wantirna Reserve and the privately owned property to the southwest.

1.2 Objective

The primary objective of the Phase 1 ESA was to review the site history of the Reserve, assess the extent of the landfilling and identify potential sources and types of contamination, in association with former landfilling activities.

1.3 Scope

The scope of works comprised a desktop assessment, site inspection and landfill gas monitoring at identified likely emission locations. The ESA included the review of the following:

- Historical aerial imagery of the site and surrounds,
- Historical titles and site plans,
- Interviews with long term site users;
- Relevant documentation held by the VIC EPA or other Authorities,
- Site-specific environmental setting,
- Former landfill-specific potential contaminants,
- Discharges to environment and associated pollution controls and performance monitoring,
- On-site and/or nearby receptors;
- A site inspection and monitoring at potential areas of landfill gas emissions, carried out by suitably qualified AECOM personnel;
- Assessment of the site as per EPA Publication 1671 Local Council Self-Assessment Tool for Closed Landfill Environmental Risk (February 2018).; and
- Preparation of a Phase 1 report (this report).

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2.0 Site Description

2.1 General Site Description

2.1.1 Landfilled Area Description

The locations of the two separate tip areas, as presented on historic Melways maps and the potentially landfilled areas, identified from historical aerials are presented in **Figure 2** (Appendix A). Copies of the two Melways Maps from 1970 and 1971-1972 are presented in Appendix F and historical aerials in Appendix C. For the purpose of this investigation, the two potential landfilled areas are referred to as the North East Reworked/Landfilled Area (NE Landfilled Area) and South West Reworked Area (SW Reworked Area) below.

Descriptions of the landfilled areas are summarised below:

- NE Landfilled Area: covers approximately 32,000 m² and is located 150 m north of Mountain Highway, between the Reserve entry road and the western boundary of the current oval field. The area is approximately 260 m in length and a maximum of 180 m wide at the largest extent.
- SW Reworked Area: covers approximately 11,500 m² and is located approximately 80 m north of Burwood Highway between the private pond waterbody and the overhead power lines. The area is approximately 160 m in length and 130 m in width. This area is private property and not part of the Reserve.

2.1.2 Wantirna Reserve Site Layout

The Reserve is a large public park and recreation area with sports and recreational facilities.

The Reserve is accessed by an unnamed road entering from Mountain Highway to the south. A roundabout approximately 100 m north of Mountain Highway provides access to all facilities.

Within the NE area, in the southern portion of the Reserve are ten (10) tennis courts, tennis clubrooms and playground area. Two carpark areas are located to the north and west of the club. East of the tennis club is a Scouts Hall, small carpark and a public playground. North of the playground is a cricket and football oval with a small pavilion on the western boundary and batting cages to the northern oval boundary. A Stormwater Pond is located directly northwest of the current cricket oval.

Further northwest of the oval is a grassed and vegetated area with a northeast-southwest aligned walking and cycling path (Eastlink Trail). Dandenong Creek flows to the west of the Reserve and Dandenong Creek Trail follows the creek alignment in a north-south direction before turning west in the western portion of the site. Dense vegetation occurs in the western portion of the site adjacent to Dandenong Creek.

A low-lying grassed and marsh area triangular in shape and small wetlands are present in the southwestern portion of the Reserve. Immediately adjacent to the southwest is the SW Reworked Area, which intersects a privately owned commercial haulage and transport building and part of a Private Pond.

Wantirna Reserve area is shown in Figure 1, Appendix A.

2.2 Site Location and Ownership

The Reserve is located at the corner of the Mountain Highway (State Route 28) and the Eastlink Tollway (M3), Wantirna, Victoria, Australia (refer to **Figure 1** – **Appendix A**). The area covered by the assessment consists of Crown Allotment 2202, Parish of Scoresby. The site details are summarised in Table 1 below.

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Table 1 Site Identification

Торіс	NE Landfilled Area	SW Reworked Area
Site Location	61 Mountain Hwy, Wantirna, Victoria	1 Burwood Highway, Wantirna, Victoria
Current Land Use	Public Park and Recreational facility (Tennis, Cricket, Football, Walking, Cycling)	Commercial Property – Haulage and Transport
Land Use Zoning	Public Park and Recreation Zone (PPRZ)	Urban Flood Zone (UFZ)
Legal Property Description	Standard Parcel Identifier: 2202\PP3478 Parent Title Volume: 11705 Folio 190 Crown Description: Allot. 2202 PARISH OF SCORESBY	Not reported
Site Ownership	Crown Land	Private
Approximately Site Area (m ³)	NE Landfilled Extent: 32,000 m ²	SW Reworked Extent: 11,500 m ²

Current and historical site information, including aerial photographs, were obtained from Lotsearch (**Appendix C**). Zoning information was obtained from the Department of Environment, Land, Water and Planning online website (http://www.dtpli.vic.gov.au/planning) on 26 July 2019 and is presented in **Appendix D**. Current Reserve ownership information was obtained from the current certificate of title by KCC in **Appendix B**.

2.3 Surrounding Land Use

The Reserve is situated in the local government area of Port Phillip, within an area of mixed land use, including commercial, residential, roadways and parkland or reserve areas.

Current land use surrounding the Reserve is summarised in Table 2 below:

Table 2 Surround Reserve Land Use

Directions	Land Uses surrounding the Wantirna Reserve
North	To the north of the Reserve the public park and recreation area extends north to Boronia Road (State Route 36).
South	The site extends to Mountain Highway, which is in turn bordered by the Melbourne School of Theology and Ambulance Victoria facilities. This is followed by Burwood Highway and park and recreation fields.
East	To the east of the Reserve is the Eastlink Tollway (M3), followed by residential properties.
West	Dandenong Creek followed by Morack Public Golf Course borders the majority of the Reserve, while in the south west boundary adjacent to the Tennis Courts is a private garden nursery, one rural zoned residential property and commercial buildings (in vicinity of SW Reworked area).

2.4 Planning Scheme and Planning Policy Considerations

2.4.1 Zones and Overlays Affecting Site

Pursuant to the Victorian Planning Scheme, the Reserve is situated within a Public Park and Recreation Zone (PPRZ). Further details can be found in the Lotsearch report in **Appendix C** and the council property planning report in **Appendix D**. The zones and overlays affecting the site are listed below in Table 3.

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Table 3 Site Zones and Overlays

Planning Type	NE Landfilled Area	SW Reworked Area
Planning Zone	Public Park and Recreation Zone (PPRZ)	Urban Floodway Zone (UFZ)
	Public Conservation and Resource Zone (PCRZ)	
Planning Overlay	Environmental Significance Overlay - Schedule 2 (ESO2)	Environmental Significance Overlay - Schedule 2 (ESO2)
	Land Subject to Inundation Overlay (LSIO)	Land Subject to Inundation Overlay (LSIO)
		Public Acquisition Overlay (PAO2)

The Planning Scheme indicates that the Reserve is currently not subject to an Environmental Audit Overlay (EAO), however, the local Council may require an environmental audit in the future as part of a planning or building permit for sensitive land use.

2.4.2 Planning Overlays Surrounding Reserve

Planning overlays of note surrounding the Reserve within a 1 km buffer radius include:

- A Vegetation Protection Overlay Schedule 1 (VPO1) is located approximately 40 m southwest of SW reworked area; and
- A Special Building Overlay (SBO) is located both 100 m east and 250 m northeast of the NE landfilled area, and 230 m west of the SW reworked area.

2.4.3 Zoning Surrounding Reserve

The land immediately southwest of the Reserve entry road is zoned a Rural Living Zone (RLZ). Land to the east and south of the Reserve is zoned Road-Category 1 (RDZ1), and south of Mountain Highway is a Commercial 2 Zone (C2Z). A Public Use Zone – Service and Utility (PUZ1) is located southwest of the Reserve, south of Burwood Highway.

2.4.4 Cultural Heritage Sensitivity

The Lot Search Report indicates that the Reserve and surrounding site is within an area of Cultural Heritage Sensitivity as specified in Division 3 of Part 2 in the Victorian Aboriginal Heritage Regulations 2007.

Further details can be found in the Lotsearch reports in Appendix C.

2.4.5 Bushfire Prone Areas

The Reserve has been identified as being within a designated bushfire prone area and therefore special bushfire construction requirements will apply.

Further details can be found in the property planning reports in Appendix C.

2.5 EPA Register Search

2.5.1 EPA Priority Sites Register Search

The Priority Sites Register lists sites for which EPA Victoria has requirements for active management of land and groundwater contamination (EPA Publication 735 EPA Contaminated Site Information Systems Priority Sites Register, December 2000). Appropriate clean-up and management of such sites is an EPA priority and, as such, EPA has issued 'Clean Up' or 'Pollution Abatement' notices on the occupiers of such sites.

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A search of the Priority Sites Register was undertaken on 8 July 2019 (EPA Victoria). The results of the search indicated that as of 8 July 2019, the Reserve is not listed on the Priority Sites Register and is not within the vicinity of a site listed on the register.

It should be noted that the Priority Sites Register does not list all sites known to be contaminated in Victoria, and a site should not be presumed to be free of contamination if it does not appear on the Priority Sites Register. However, since the Reserve does not exist on the EPA Priority Sites Register, no active management plan is required for the Reserve.

2.5.2 Certificates and Statements of Environmental Audit

A search on EPA Interaction Portal for properties issued with a certificate or statement of environmental audit as part of a 53X audit or with a report as part of a 53V audit, under part IXD of the Environment Protection Act 1970 was undertaken on 8 July 2019 (**Appendix C**). The Reserve is not listed on a certificate or statement of environmental audit. However, the Reserve is within 1 km of 1 property that has previously been issued with a certificate or statement of environmental audit. This audit site, Victorian EPA CARMS No. 72881-1, is located approximately 350m to the southeast of the Reserve at 56 Mountain Highway, Wantirna. Site investigation was undertaken between 2013 and 2015. The audit site was previously used as a market garden from 1954 to 2004 and for Eastlink Construction Depot use from 2004 to 2006. Groundwater quality was not assessed with groundwater not encountered within 20 m of ground surface in Silurian siltstone. Minor small amounts of remnant asbestos containing material and inert solid waste items may be present in soils. It was determined that the site was suitable for Low Density Residential or Urban Residential and Public Open Space as defined in the National Environmental Protection Measure (NEPM) guidelines.

2.5.3 Groundwater Quality Restricted Use Zone (GQRUZ)

The SEPP Waters of Victoria establishes a framework for the prevention and management of contamination of groundwater in Victoria. As part of this, the SEPP incorporates the use of groundwater quality restricted use zones (GQRUZ) to function as a tracking and information tool, which can be applied when groundwater is not suitable for particular beneficial uses, due to contamination. A search of the EPA database conducted by Lotsearch (**Appendix C**) reported no GQRUZ is located within 1 km of the site.

2.5.4 Registered Waste Management Facilities & Landfills

A search on the EPA Victorian Landfill Register for properties registered as landfill sites was undertaken by Lotsearch on 8 July 2019 (**Appendix C**). The search reported one site approximately 300 m to the southwest of the site that is currently closed. This site was reported on the Melways 1970 and 1971-1972 maps as the 'City of Nunawading Tip' and appears to be closed in the 1986 Melways Map, replaced with a 'City of Nunawading Waste-Transfer and Recycling Centre'. A search of the Statewide Waste and Resource Recovery Infrastructure Plan Facilities returned one facility located at the closed landfill above, the Whitehorse Recycling & Waste Centre, which is operated by the Whitehorse City Council since approximately 1998.

Neither of the two 'City of Knox Tip' sites appear in the EPA Victoria Landfill Register.

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3.0 Site Environmental Setting

3.1 Topography

Based on the topography of the site, the site slopes to the northwest towards Dandenong Creek. A low-lying flood plain is situated to the west of the entrance roundabout and extends towards the SW Reworked Area. The area of the NE Landfilled area is undulating in profile. The carpark area is slightly elevated above the surrounding grassed areas.

The elevation contours provided by LotSearch (**Appendix C**) appear to be based on survey data between 1970 and 1985 and display unnatural topography within the two reworked areas (NE Landfilled Area and SW Reworked Area).

3.2 Sensitive Receptors

Sensitive receptors located within a 1 km radius of the site include:

- Rural residential housing immediately southwest and residential housing 400 m east of the site.
- Dandenong Creek located immediately adjacent to the north western site boundary.
- Wetlands in low-lying areas in the western portion of the site; and
- Public recreational facilities located on the site.

3.3 Geology

Regional geology of the site was obtained from GeoScience Victoria (GSV), Earth Resources through the GeoVic online portal (a division of the Department of Economic Development, Jobs, Transport and Resources). According to the Seamless Geology Spatial Dataset 2014 (Scale 1:50,000), accessed on 23 July 2019 (https://earthresources.vic.gov.au/geology-exploration/maps-reports-data/geovic), the geology underlying the site is identified as:

- Quaternary aged alluvium (Qa1); and
- Silurian aged 'Anderson Creek Formation' (Sxa) comprising massive siltstones interbedded with thin sandstones and minor conglomerate.

The site specific information is described below in Section 4.4. At least 0.6 m thick clay to silt cover layer over the historic landfill was reported in the NE Landfilled Area.

3.4 Hydrogeology

Information and data on the regional hydrogeology relevant to the site have been obtained from the Department of Environment, Land, Water and Planning (DELWP) website (https://www.water.vic.gov.au). The Groundwater Resource Report indicated the site comprises of the following groundwater aquifers / aquitards:

- QA Quaternary Aquifer comprises sand, gravels, clay, silts and extends to 7 m bgl with total dissolved solids (TDS) concentrations ranging from 1,001 to 3,500 mg/L; and
- BSE Mesozoic and Palaeozoic Bedrock (basement) sedimentary (fractured rock): Sandstone, siltstone, mudstone, shales. Igneous (fractured rock): includes volcanics, granites, granodiorites. These extend from 7 to 207 m bgl with TDS locations ranging from 1,001 to 3,500 mg/L.

Based on Leonard (2006), hydraulic conductivities of the Palaeozoic Bedrock vary depending on the degree of fracturing and/or weathering and are generally low, between 0.001 m/day and 0.3 m/day. As described in Section 4.4, in the site-specific bores the reworked fill was underlain by clays suggesting low permeability of the sediments underlying the site.

According to the groundwater resource report, the depth to groundwater is generally less than 5 m bgl. As described in Section 4.4, groundwater was encountered at approximately 3.0 m below ground level (m bgl) in the NE Landfilled Area. Based on the site walkover observations there are significant marsh C:\Users\mark.s.davidson\Documents\Work Documents\Wantima Reserve Phase 1 Assessment 2019_rev1_draft_20190813 Rev1.docxP:\docSX160548185\6. Draft Docs\6.1 Reports\Wantima Reserve Phase 1\Wantima Reserve Phase 1 Assessment 2019_rev1_draft_20190716.docx Rev1_draft_20190716.docx Revision 1 – 16-Aug-2019

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and swamp areas within the site near Dandenong Creek suggesting a very shallow water table in the vicinity of the site. TDS concentrations were noted to range between 1,001 to 3,500 mg/L, which classifies the groundwater across several segments of the State Environment Protection Policy (SEPP Waters) (2018) and would conservatively classify the site as Segment A2.

The Visualising Victoria's Groundwater map (https://www.vvg.org.au/vvg_map accessed 12 August 2019) indicates the salinity in the area is 3,501 to 13,000 mg/L. This would classify the site as Segment C under SEPP Waters (2018).

The protected beneficial uses of the most conservative segment (Segment A2) include: Water Dependant Ecosystems and Species, Potable Water Supply (acceptable), Potable Mineral Water Supply, Agriculture and Irrigation (irrigation and stock watering), Industrial and Commercial, Waterbased Recreation (primary contact recreation), Traditional Owner Cultural Values, Cultural and Spiritual Values, Buildings and Structures and Geothermal Properties. It is considered that groundwater beneath the site is unlikely to be suitable for water supply, based on background TDS and therefore we have adopted Segment B for use in assessing landfill risk (Section 6).

Based on the site location and current usage, Water Dependant Ecosystems and Species represent the most realistic protected beneficial use at the site.

A copy of the groundwater resource report from DELWP and image of the VVG salinity map is provided in Appendix E.

3.5 **Registered Groundwater Search**

A search for registered groundwater users located within a 2 km radius of the site was undertaken by Lotsearch, using the Water Measurement Information System (WMIS) maintained by DELWP.

The results indicate that there were approximately 190 registered groundwater bores within a 2 km radius of the site. Given the large number of groundwater bores in the area, the review for this ESA excluded groundwater bores registered for 'Non-Groundwater' purposes. A total of 18 registered groundwater bores are located within 2 km of the site, three (3) of which are registered for groundwater investigation purposes and 15 for domestic and stock purposes. It should be noted that the total depth of all bores was less than 15 m and therefore are unlikely to be used for domestic supply.

Bore ID	Date Completed	Depth (m)	Direction	Distance from Centre (m)	Registered Use
WRK960389	22/11/2002	6	SW	238	Investigation
WRK960388	22/11/2002	6.4	SW	248	Investigation
WRK960390	22/11/2002	6	SW	252	Investigation
WRK981922	11/7/2007	5	SW	556	Domestic & Stock
WRK981923	10/7/2007	8.5	SW	556	Domestic & Stock
WRK981924	11/7/2007	10	SW	556	Domestic & Stock
WRK981925	11/7/2007	5	SW	556	Domestic & Stock
WRK981926	12/7/2007	5.5	SW	556	Domestic & Stock
WRK981927	12/7/2007	13	SW	556	Domestic & Stock
WRK981928	13/7/2007	15	SW	556	Domestic & Stock
WRK981930	12/7/2007	5.5	SW	556	Domestic & Stock
WRK981931	12/7/2007	13	SW	556	Domestic & Stock
WRK981932	13/7/2007	15	SW	556	Domestic & Stock
WRK981933	10/7/2007	10.5	SW	556	Domestic & Stock

			_			
Table 4	Registered	Groundwater	Bores	Search	(2 km)	WMIS
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Bore ID	Date Completed	Depth (m)	Direction	Distance from Centre (m)	Registered Use
WRK981934	13/7/2007	15	SW	556	Domestic & Stock
WRK981935	10/7/2007	8.5	SW	556	Domestic & Stock
WRK981936	11/7/2007	10	SW	556	Domestic & Stock
WRK981937	13/7/2007	15	SW	556	Domestic & Stock

3.6 Hydrology

The nearest surface water body is Dandenong Creek, which flows in a southwest direction and bounds Wantirna Reserve immediately to the west and northwest of the site.

A stormwater pond is located directly north of the current cricket oval and collects surface water from the drains adjacent to the cricket batting cages located northwest of the oval. There is an extensive stormwater drainage network associated with the sporting grounds. Surface water is diverted from the former landfill area, towards a storage dam in use to the northwest of the current oval.

According to, SEPP Waters (2018) surface water at the site is classified as Urban (the areas within the urban growth boundary for Metropolitan Melbourne), which includes Dandenong Creek, the tributaries of the Yarra, Maribyrnong and Werribee Rivers, and the current developed areas in the Mornington Peninsula and Western Port catchments. This classifies the site as highly modified for water dependent ecosystems.

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4.0 Historical and Background Information Review

4.1 Current and Historical Title Review

Current and historical certificates of title for the Reserve only and not the private property in the SW Reworked area were provided by KCC, which were obtained from Feigl & Newell Title Searchers Pty Ltd for the property address 61 Mountain Highway, Wantirna, Victoria. The current title describes the property as Volume 11770 Folio 352, under the ownership of the Crown. The certificate of title indicates that the Crown have owned all land parcels in this title since 4 January 2001 and is listed for Crown Land Temporarily Reserved for Recreation Leisure and Tourism. A total of eight (8) separate parcels have been split and/or consolidated over time to create the current title.

A summary of the certificate of title is provided in Table 5 below and certificates of title are presented in **Appendix B**.

Title Reference	Location	Period of Ownership	Proprietor
Volume 2747	All of site	31/10/1899 - 04/03/1903	Allan Williamson (Market Gardener)
Folio 250		04/03/1903	The Equity Trustees Executors and Agency Company Ltd
		04/03/1903 - 06/04/1911	Edward Ford (Butcher)
		06/04/1911 - 02/12/1930	John Sanders (Farmer)
Volume 5751 Folio 143	Derived from V. 2747, F. 250	2/12/1930 – 3/10/1940	John Sanders (Farmer)
Volume 6488 Folio 592	Derived from V. 5751 F.143	3/10/1940 — 7/05/1947	Frederick McDowall (Contractor)
Volume 7220	Derived from	7/05/1947 – 18/08/1949	Elsie McLean
Folio 886	V. 6488 F. 592	18/08/1949 – 28/06/1966	William Nest (Butcher)
Volume 8626 Folio 157	Derived from V. 7220 F. 886	28/06/1966 - 27/09/1977	The President Councillors And Ratepayers Of The Shire Of Knox
		27/09/1977 - 13/03/1996	Melbourne Metropolitan Board of Works
		13/03/1996 - 30/11/2001	Melbourne Parks & Waterways
		30/11/2001 - present	Transferred to Crown
Volume 4676	Derived from	15/03/1923 - 23/10/1936	Archibald Sanders (Orchardist)
Folio 112	V. 2747, E 250	23/10/1936 - 31/08/1944	Norman Taintor (Orchardist)
		31/08/1944 - 16/07/1974	Frederick Finger (Orchardist)
		16/07/1974 - 10/10/1984	Country Roads Board
Volume 9585 Folio 004	Derived from V. 4676,	10/10/1984 — 13/03/1996	Melbourne And Metropolitan Board of Works
	F. 112	13/03/1996 - 30/11/2001	Melbourne Parks & Waterways
		30/11/2001 - Present	Transferred to Crown
Volume 5030 Folio 844		03/09/1925 – 27/09/1977	The President Councillors And Ratepayers Of The Shire Of Fern Tree Gully

Table 5	Table of Historical and Current Certificates of Title
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Certificate of Title Reference	Location	Period of Ownership	Proprietor
	Derived from V. 2747, F.	27/09/1977 – 13/03/1996	Melbourne And Metropolitan Board of Works
	250	13/03/1996 – 30/11/2001	Melbourne Parks & Waterways
		30/11/2001 - Present	Transferred to Crown
Volume 11770 Folio 352	Derived from V.8626 F. 157, V. 9585 F. 004 and V. 5030 F. 844 (and other parcels outside of investigation area)	4/12/2001 - Present	Crown Land Temporarily Reserved for Recreation Leisure And Tourism

According to the certificate of title, Volume 8626 Folio 157 was transferred to the ownership of The President Councillors And Ratepayers Of The Shire Of Knox on 26 August 1966 and then transferred to the Melbourne Metropolitan Board of Works on 27/09/1977. This is consistent with the time period of the NE Landfilled Area operating under the ownership of KCC.

4.2 **Historical Aerial Photography**

A search of relevant historical aerial photographs was undertaken. Nine historical aerial photographs were reviewed for the site from the period 1951 to 1991 and two Google Earth images were reviewed from 2005 to 2009. The information obtained during the review for a selection of significant aerial photographs is summarised in Table 6 below. Copies of all of the aerial photographs and Google Earth images obtained are attached in Appendix C.

Table 6 Aerial Photograph Review

Date	Description
1951	Reserve and Surrounds Predominantly vacant with grass likely used for agricultural purposes (grazing). Tree cover present along Dandenong Creek. Two tennis courts in the location of current tennis club and faint circular field visible to north of courts. A small structure is also located on the site immediately to the north of the site. The use of this structure is not clear. North of the field appears to be NE/SW aligned natural drainage channels. The area surrounding the site is sparsely populated and appears to be dominated by agricultural land with some orchards. Several small structures, potentially residential, are located southwest of the site long Burwood Highway.
1960	Reserve Unsealed road runs N/S through the middle of the site, developed along the alignment of the power lines also running N/S. Some development extended beyond the boundary to the north and north east. New orchards developed to the east of the tennis courts and cricket oval. Surroundings New development to the south west of the Reserve appears to be visible with reworked surface cover along the section north of Burwood Highway.
1962	<u>Reserve</u> Remains vacant and unchanged since the 1960 aerial photograph. <u>Surroundings</u>

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Date	Description
	Tear drop shaped dam constructed to the immediate west of the reserve and south of the Dandenong Creek tree cover in the vicinity of the SW Reworked Area. Additional reworked surface cover west of this area in the 1960 aerial photography is present, which is consistent with groundworks for site development. A minor track appears to link the dam with the reworked area, which may be the origin of the material. Some tree cover to the north of the dam appears to have been cleared.
1963	<u>Reserve</u> Remains vacant and unchanged since the 1962 aerial photograph. <u>Surroundings</u> Tear drop shaped dam completed to the immediate west of the reserve.
1966	ReserveRemains vacant and unchanged since the 1963 aerial photograph.SurroundingsMajor dam excavation in the south-western portion of the SW Reworked Area, replacing the tear- shaped dam. Area SE of the excavation appears to be reworked / holding excavated material. No evidence of other filling suggesting municipal waste disposal is apparent. The area to the east of the SW Reworked Area appears to have been levelled.
1967	ReserveSome reworked material appears showing signs of stockpiling.SurroundingsMajor dam excavation in the south-western portion of the SW Reworked Area is complete. AreaSE of the excavation appears to be similar to the 1966 aerial photograph.New buildings present to the west of the original oval, potentially residential.
1968	Reserve Reworked material still visible to the north of the cricket oval and a distinct unsealed track to the north of the cricket oval. Surroundings Development of several buildings west of the cricket oval with some defined roads and vehicles. The area north of these buildings appears to have been reworked but appears to be consistent with levelling of the site and no evidence of other filling suggesting municipal waste disposal is apparent. Ground disturbance appears to occur in the location of the stormwater pond, east of the NE Landfilled Area and north of the orchards. The small triangular pond further east of the stormwater pond area is completed.
1970	Reserve Unsealed road developed to the west of the cricket oval providing access to the triangular shaped area where landfilling appears to be occurring. Defined tracks and material stockpiling is visible suggesting municipal waste disposal and infilling. Surroundings The stormwater pond is completed to the east of the NE Landfilled Area. A small area to the west of the NE Landfilled Area appears to have been levelled, but no evidence of stockpiling or landfilling is apparent.
1975	Reserve The original oval has been removed and appears to have been levelled with fill. It is uncertain whether landfilling occurred in this area between 1970 and 1975. A new cricket oval has been constructed in the area where the NE Landfilled Area was in the 1970 aerial photograph. The area surrounding the oval appears to be still under construction. Surroundings

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Date	Description
	Infrastructure including three small ponds is constructed west of the Dandenong Creek.
1978	Reserve The cricket oval appears to be complete and a new pavilion building constructed on the western side of the oval. Additional stockpiles appear to the north and south of the oval, consistent with new filling for earthworks. Carparking and two additional courts has been developed north of the tennis club and courts. Surroundings Other than the development of a golf course to the west of Dandenong Creek, no changes to the surrounding land since the 1975 aerial photograph.
1991	<u>Reserve</u> Development of a second oval to the east of the oval in the 1978 aerial photograph, with a new cricket building on the western boundary of the second oval. Development of the new entrance road from Mountain Highway, two roundabouts and the carpark south of the oval in the 1978 aerial photograph. <u>Surroundings</u> No changes to the surrounding land since the 1978 aerial photograph.
2005	Reserve Additional vegetation cover in western portion of the site in the low-lying wetland area. Development of the 1978 oval into recreational park land. Two additional tennis courts developed north of the tennis club building. Surroundings Gravel cover in the area of the SW Reworked Area and development of the area to the southwest into what appears to be commercial property.
2009	<u>Reserve</u> Building to west of 1975 oval is removed. No other changes to the 2005 aerial photograph <u>Surroundings</u> New large square industrial building constructed in the area of the SW Reworked Area.

Based on the findings of the aerial photographs, the NE Landfilled Area appears to have been used as a landfill between approximately 1968 and 1975. No evidence of landfilling was observed in the vicinity of the SW Reworked area, although disturbed areas consistent with site development and construction were evident.

4.3 Site History

Design plans were provided by KCC for the Reserve for 1965, 1967 and 1979. A review of these plans is summarised below in Table 7 and copies of the plans are provided in Appendix F.

Table 7 Wantirna Reserve Design Plans

Year	Development Plans
1965	The plan titled 'Development Plan' displays the original oval location to the south of the site and two proposed ovals; one 'Proposed Main Oval' in the area of the original oval and a 'Proposed Secondary Oval' to the north in the area of the NE Landfilled Area. A new road to the west of the ovals and an area reserved for a pavilion between the two ovals was also proposed. Topographic contours and marker on the map indicate an open watercourse in the area of the 'Proposed Secondary Oval' was present running from the east to the west of the site.

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Year	Development Plans
1967	The plan titled 'Proposed Garbage Disposal Area' was outlined the location of the NE Landfilled Area overlying half of a 'Proposed Future Oval' in the same location as the 1965 main oval plan. The proposed area for disposal is consistent with the area in the 1970 aerial photograph. A new entrance road with topographical drainage to the west of the ovals, a drainage easement to the northwest of the ovals flowing to Dandenong Creek and an open drain on the eastern boundary of the garbage disposal area was proposed. A proposed grade of 1 in 200 from South to North and 1 in 80 from the centre of the landfill area to the eastern and western boundaries was proposed.
1979	The plan titled 'Existing Conditions – July 1979' indicate the site had tennis and basketball courts, a clubhouse and small crushed rock car park in the area of the present tennis club facilities. An existing oval and pavilion on the western side in the location of the NE Landfilled Area is present. An open drain along the access track to the west of the oval connect Mountain Highway to the pavilion. The drain appears to move to the northwest towards Dandenong Creek. There appears to be 'New work in progress' to the north of the oval, consistent with the reworked area and stockpiled material in the 1975 aerial photograph (outlines are provided on Figures F2 and F4 , Appendix A).

4.4 Previous Environmental Investigations

No previous environmental investigations have been made available to AECOM concerning this site. However, a geotechnical report conducted by A.S. James Pty Ltd (A.S. James) in 2014 was made available to AECOM. In the report, dated 24 March 2014, a geotechnical investigation of the current oval, area directly north of the tennis club Courts 6 -10 and in the vicinity of the oval present between approximately 1975 and 1991 was completed to inform the design for upgrades to the site. The location of these boreholes and the extent of previous fields in presented in **Appendix F**. The findings from a total of 20 boreholes across the site included:

- Sandy silt between 0.1 and 0.6 m thick, encountered where historical landfilling occurred in the NE Landfilled Area, while crushed rock fill (road base) was encountered in the area of the current carpark north of the tennis courts (south of the NE Landfilled Area).
- Waste was encountered between approximately 0.1 and 4.5 m bgl in the NE Landfilled Area. The material classified as historic landfill comprised grey brown/brown grey mottled clays with cloth material, plastic, gravels, metal, glass and brick. No indication of waste percentage was reported although waste was classified as accessory materials.
- A perched groundwater aquifer was encountered at approximately 3.0 m below ground level (m bgl) at two boreholes in the NE Landfilled Area. Three boreholes in the vicinity of the 1975 -1991 oval in the NE Landfilled Area were terminated before 3.0 m bgl, however it was noted to be very moist at the base (approximately 3.0 m bgl). No groundwater was encountered during the investigation.
- It was concluded that due to the unknown and uncontrolled nature of filling that the soil could have the potential to be contaminated.

In several bores natural soils were encountered, comprising primarily of clays.

Materials described as waste was reported in bores BH1-BH4, BH10 and BH11, all located within the NE Landfilled area, however waste of similar thickness was also described in bores BH5 – BH7, BH11, BH12 and BH14 located to the south of it (carpark and vegetated area north of the tennis courts) in the area of the original (1951-1970) oval. This suggests that historical landfill could have extended further south of the NE Landfilled Area, or that landfill marginal materials were used while relandscaping the original oval area between 1970 and 1975.

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Waste thickness of at least 4 m was adopted to estimate the volume of waste filled for the EPA landfill assessment tool.

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5.0 Site Reconnaissance

5.1 Site Walkover

A site walkover was conducted on 8 July 2019 by AECOM. The site walkover consisted of a visual assessment of the surface topography, drainage and surface water receptors, including subsidence, differential settlement, capping and vegetation conditions on site.

No visual observations of waste impact during the site walkover was noted, with no exposed waste, odours, surface cracking or obvious displacement/subsidence. No leachate impact was observed at any of the water bodies at the Reserve. No seepage was observed and visual observations indicate the Stormwater Pond to the northwest of the cricket oval does not appear affected by leachate (i.e. no odour, nutrient-rich black or green water).

Drainage channels were observed across the site with open drainage channels and stormwater grates associated with the cricket oval and recreational walking areas park drainage.

The site walkover was conducted after significant rainfall and therefore the monitoring of identified locations could not be completed in the same mobilisation. Surface water was observed on site following the rainfall. All locations identified as likely landfill gas (LFG) emission locations were marked for the subsequent monitoring event.

5.2 Gas Monitoring

The LFG emissions survey was undertaken on 7 August 2019 by AECOM. The purpose of monitoring was to ascertain if any methane emissions occur through the former landfill cover or penetrations through it, and potentially migrating through the subsurface services at the site and in the immediate vicinity.

Monitored locations included buildings, service and drainage pits and other subsurface features marked during the initial site inspection in July 2019. Where additional service pits or features potentially penetrating the cover were identified, these were added to the list during the emissions survey. Other locations included edges of concrete slabs and bike / footpath, noticeable ground cracks, areas of low / cut grass, and other potential cover penetrations such as sign posts, fence posts, wooden posts (bollards) marking the roads or carpark boundaries; coordinates of the selected features only were recorded. Tennis Club buildings and playground locations were not investigated due to lack of access however they are remote from the identified fill extent. Other buildings were monitored outside, with readings taken along the concrete foundation edge / cracks, subfloor vents or pipework penetrating the ground.

Locations are presented on Figure 3, Appendix A.

Monitoring was undertaken in general accordance with Landfill Gas Fugitive Emissions Monitoring Guidelines – Publication 1684 EPA, 2018). Emissions at each location were measured for 30 to 60 seconds using a low concentrations methane detector (Inspectra Laser).

Ground conditions across the investigated area appeared moist but not boggy, with the apparent cracks relatively open. Soil did not appear to be saturated, however it is possible that available unsaturated gas migration pathways may have been reduced due to clay swelling.

5.2.1 Meteorological Conditions and Atmospheric Pressure

Weather observations including rainfall, temperature and pressure were downloaded for the Scoresby Station for the three days prior to monitoring.

Weather conditions were favourable for gas emissions monitoring. Gas emissions monitoring was conducted following over 3 days of falling barometric pressure conditions and minor precipitation as recorded at BOM station 086077 Moorabbin Airport (refer chart below). At the nearest weather station recording rainfall data (Glen Waverley Golf Course, Station 86303), data for August was not available at the time of preparation of this report. Significant daily precipitation (over 10mm) was reported on 14-16 July, 24 July and the latest on 27 July. During the monitoring the weather was calm and overcast.

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Figure 1 Weather at Moorabbin Airport Week Prior to Site Assessment

5.2.2 Methane Emissions Monitoring Results

Tabulated results of field measurements are presented in Appendix G.

Methane concentrations recorded in ambient air varied between 0.3 and 1.5 ppm.

Methane concentrations recorded off the ground, cracks and surface penetrations such as bollards, sign posts and fence posts varied between 0.8 and 1.9 ppm. Those recorded at the buildings, structures and service / drainage pits varied between 0.7 and 3.9 ppm. All methane concentrations recorded during the emissions survey were consistent with ambient background, suggesting no emissions from the former landfill.

There were no areas with apparent distressed vegetation that may have been caused by soil oxygen depletion. There were no odours or effervescence observed.

In conclusion, the recorded methane emissions and observations made during the gas emissions survey suggested no methane emissions from the former landfill at Wantirna Reserve and therefore the potential LFG hazards appear low.

However, if future site development involves disturbance of the cover, such as resurfacing or excavations or deep service trenches, it is recommended that LFG hazards are addressed as part of OHS, e.g., using of monitoring equipment such as Lower Explosive Limit (LEL) meter and risk mitigation procedures.

Furthermore, if methane above LEL is recorded at the site of potential building(s), building control measures have to be considered after the risk assessment in accordance with EPA guidelines and international standards (e.g., BS 8485).

5.3 Local Accounts

Tennis Club Members

A site inspection and interview with Mr Daryl Barrett (Tennis Club Member) and Mrs Alison Rogers (Tennis Club Member and Council Contact), was carried out on 22 July 2019 by AECOM and representatives from KCC. It is noted that both Mr Barrett and Mrs Rogers have been members of the club since approximately 1980, and their knowledge of the operation of the site was limited to the final years and closure of the landfill, however no longer term members were available at the time of the

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interview. A marked-up plan indicating the extent of the landfill from their memory has been provided in **Appendix D**.

A record of the Tennis club history was provided by Mr Darryl Barrett detailing the development of the site. In 1993 when a light pole to the north of the court 8 was constructed, rubbish and debris were observed at approximately 2 m below ground level. No other refuse was encountered at the tennis club.

An account of the history of the tennis club site was provided by a member of the tennis club to KCC. It detailed that the site was proclaimed a Reserve in 1925 and that the tennis club was built shortly after, with the Wantirna Club commencing in 1934 at the site. It was noted that Melbourne Metropolitan Board of Works (South East Water) took over the lease of the area from KCC in approximately August 1986.

Knox Waste Collection Contractor

A recount of the waste collection for Knox City Council was provided by Richard Lever to KCC on 24 July 2019. He advised that in the KCC area from pre 1950s to approximately 1968 waste collection was carried by a contractor (Cook). This was a pay-per-use service, rather than KCC owned. In 1968 a company (Vantiller and Scott) collected in the area until approximately 1975. Cleanaway then commenced collection. The domestic collection service was not large as the as the area was predominantly orchards still.

At Wantima Reserve, Koomba Road was possibly the name of the access road to the old tip and from approximately 1975 onwards the area was referred to as the 'old tip'. Mr Lever commented that signage was visible on this old access road in the mid 1970s.

He did not recall the tip itself, but that that Vantiller did tip waste there during it's time collecting waste.

Knox Staff Member involved in Reserve Upgrade Works

A recount of redevelopment of the access road and carparks at the site was provided by a KCC staff member on 203 July 2019. He advised that waste was encountered at approximately 0.5 m bgl at the roundabout west of the carpark. Waste encountered comprised significant volumes of rubber fan belts in and unidentified liquid. No other information could be ascertained.

The location of the roundabout and carpark is outlined on Figure 1 (Appendix A).

Cricket Club Member

A recount of the site history was provided by Bernie Whitney, a cricket player at the Reserve since the late 1960's on 30 July 2019. The original cricket ground was located in the '1951- 1970 oval' location outlined in Figure 2 (Appendix A). He advised that the landfill was located to the north of the original ground in the '1975-1991 Oval' location.

He recalled that the site was a Council tip, but that the public could drive in with trailers to dispose of rubbish and it was freely open. The landfill emitted a strong odour on hot days.

An oval was constructed in the area where the landfill was, and the Council intended to lay turf on the oval, which club members opposed. Therefore, the oval was top dressed and if work was completed at the ground, paper and rubbish were encountered.

The ground experienced variable settlement "like a potato chip" during the period it was used for recreational purposes.

Mt Whitney recalled a deep guttering adjacent to the Reserve's main drive entry (the drive to the tip) always filled with a significant amount of water during rainfall events.

Summary

In conclusion, based on the anecdotal evidence, municipal waste was accepted at the former landfill north of the tennis courts apparently within the NE Landfilled Area. However, other waste may have been accepted, or deposited later during the redevelopment of the site and relocation of the ovals. Also, potentially reworked fill from the margins of the landfill may have been used south of the NE Landfilled Area.

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The volumes of the waste are anticipated to be relatively low due to limited rural population in the region at the time of landfilling.

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Landfill Risk Score - EPA Pub. 1671 6.0

Self-Assessment Tool 6.1

For the purpose of informing planning decisions with regards to closed landfills, the EPA Publication 1671 should be incorporated in the assessment of risks to the environment and human health. The Victorian Auditor General's 2014 report on landfill management in Victoria, recommendation number 5, states that "Councils work with the Environment Protection Authority and the regional waste and resource recovery groups to identify closed landfills, assess their risks and prioritise actions at a regional scale to address these".

The findings of this report have been assessed against the closed landfill self assessment tool and provide a rating to the NE Landfilled Area.

The SW Reworked Area is not considered in this assessment as no evidence of landfilling activities has been observed.

A summary of the outcomes of this tool are summarised in Table 8 below. The self assessment tool is attached in Appendix H.

Section		Score
1 Location Attributes		
1.1 Distance to nearest sensitive receptor	< 250 m (30m to Dandenong Creek; gathering or meeting places – on site buildings)	0
1.2 Geological sensitivity to landfill activities	Low sensitivity e.g. thick laterally contiguous clay/less fractured hard rocks -Fractured or fissured, aquifers of low to moderate productivity, geology Quaternary Alluvium and basement and Mesozoic and Palaeozoic Bedrock (basement) sedimentary (fractured rock)	3
1.3 Engineered cells	No engineered/lined cells	0
1.4 Groundwater quality (SEPP)	Segment B – the site is not used for drinking water beneficial uses.	3
	Total Score	6
2 Management Attributes		
2.1 Volume of waste filled	5,001-10,001 annually (based on estimated area 32,000m ² , approximately 5-8 years of operation (1967 – 1975), a conservative thickness of 4.0 m of waste and a tonnage conversion factor of approximately 0.3. ¹	4
2.2 Waste types accepted/licensed	Putrescible, solid inert and clean fill (based on personal communications as provided by Council that domestic refuse was deposited)	3
2.3 Gas control	Landfill is not capped (so no landfill gas control)	0
2.4 Leachate control	No leachate extraction and disposal	1
2.5 Stormwater/ surface water control	Conventional capping, surface water diversion from active areas, toe drains and storage dam in use.	3
	Total Score	11
3 Monitoring		

Table 8 **Closed Landfill Environmental Risk Assessment Tool**

¹ <u>http://www.wasteauthority.wa.gov.au/media/files/documents/GN6VoltoTonnes.pdf</u> C:Users\mark.s.davidson\Documents\Work Documents\Wantima Reserve Phase 1 Assessment 2019_rev1_draft_20190813 Rev1.docxP:\605XI60548185\6. Draft Docs\6.1 Reports\Wantima Reserve Phase 1\Wantima Reserve Phase 1 Assessment 2019_rev1_draft_20190716.docx Revision 1 – 16-Aug-2019 Prepared for – Knox City Council – ABN: 24 477 480 661

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Section		Score		
3.1 Groundwater monitoring	None	1		
3.2 Gas monitoring	None (except as part of this investigation)	1		
3.3 Leachate monitoring	None	1		
3.4 Surface water monitoring	None	0		
3.5 Cap maintenance program	Some, sufficient grounds maintenance only	2		
	Total Score	5		
4 Gas, Groundwater and Su	rface Water Risks			
4.1 Groundwater risk	Moderate	3		
4.2 Gas risk	Insignificant – based on LFG site walkover	5		
4.3 Surface water risk	Insignificant	5		
	Total Score	13		
5.0 Off-site management				
Are there community complaints regarding this site?				
Are there signs of litter beyond the boundaries?				
6 Cells, Rehabilitation and Monitoring				
6.1 Does Council know how many cells have been created at this site?				
6.2 Have there been any rehabilitation works undertaken?	Well covered (soil / grass)	4		
	Total Score	4		
	Section 1	6		
	Section 2	11		
	Section 3	5		
	Section 4	13		
	Section 6	4		
	OVERALL ASSESSMENT SCORE	39		

Based on the overall score of 39, the site falls within the >37 band, for which EPA guidelines (Pub. 1671, 2018) recommend that the site is potentially low risk based on the assessment. Consider when future reassessment should be undertaken.

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7.0 Findings

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7.1 Potential Sources of Contamination

The findings of the potential sources of contamination are summarised below:

- While earthworks appear to have been undertaken in the SW Reworked Area, no evidence
 of landfilling activities were obtained (with the exception of the historical Melways map). On
 this basis it is considered unlikely that the SW Reworked Area is impacted by disposal of
 municipal waste;
- Based on the information obtained, the NE Landfilled Area is confirmed to have operated as a municipal landfill. This area is confirmed to have relatively shallow cover (0.6m);
- No information regarding types of landfill waste was obtained, although anecdotal evidence suggested the landfill comprising primarily of putrescible waste;
- Based on anecdotal evidence landfilling may have occurred south of the NE Landfilled Area over the site of the original 1951 -1970 oval area;
- Information for the NE Landfilled Area being used as a municipal landfill indicate that the soils in the area are contaminated with waste. The soil between the NE Landfilled area and the tennis courts also has potentially contaminated soils based on the historical filling observed in aerial photographs and tennis club recounts;
- Visual observations during the site walkover noted no exposed waste, odours, surface cracking or obvious displacement/subsidence; while during the LFG walkover, no methane emissions were reported above ambient air levels;
- No seepage or impact to the beneficial uses which is classified as Urban, highly modified was observed. Visual observations indicate the Stormwater Pond to the northwest of the cricket oval does not appear affected by leachate (i.e. no odour, nutrient-rich black or green water); and
- Groundwater at the site was noted at shallow depths in the NE Landfilled Area (approximately 3 m bgl) during previous investigation by A.S James (2014), which was within the extent of the historical fill (approximately 4 m bgl). Therefore, there is a high potential for leachate generation in the landfilled area.

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8.0 Conclusions and Recommendations

Conclusions

The following conclusions are based on the information obtained from KCC and sources of information as noted in this report:

- The site historical review indicates that the NE Landfilled Area at the Wantirna Reserve was operated as a landfill area between approximately 1968 and 1975 (based on aerial photographs, Melways maps and anecdotal evidence);
- The SW Reworked area, located on private land, has evidence of reworking for drainage and nearby development only and no evidence of landfill activites;
- Soil in the NE Landfilled area and the area between the current tennis courts and the NE Landfilled area has the potential for contaminated soils to be present;
- Surface water at Reserve does not appear impacted by leachate. There is an extensive stormwater drainage network associated with the sporting grounds and surface water is collected in the Stormwater Pond, which does not show impact of leachate;
- Groundwater is potentially 3 m bgl and waste is up to 4 m bgl, therefore there is a high potential for leachate generation in the landfilled area. However, given the site has been closed for approximately 44 years, generation of leachate is likely to have reduced; and
- The recorded methane emissions and observations made during the gas emissions survey suggested no methane emissions from the former landfill at Wantirna Reserve and therefore the potential LFG hazards appear low.
- Based on the EPA Local Council closed landfill self-assessment tool, the site represents a low risk. However, future risk reassessment should not be excluded, particularly if site redevelopment involves soil disturbance.

Recommendations

- If future site development involves the excavation of material from the site or stockpiling of material from the Reserve, soil management including soil testing and classification for reuse and/or disposal should be undertaken. Stormwater management should be considered and managed accordingly on site during redevelopment to prevent potentially contaminated soil entering the waterways and Dandenong Creek though on site drainage channels.
- If future site development involves disturbance of the cover, such as resurfacing or excavations or deep service trenches, it is recommended that LFG hazards are addressed as part of OHS, e.g., using of monitoring equipment such as LEL meter and risk mitigation procedures. Furthermore, if methane above LEL is recorded at the site of potential building(s), building control measures have to be considered after the risk assessment in accordance with EPA guidelines and international standards (e.g., BS 8485).

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9.0 Standard Limitation

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It is based on generally accepted practices and standards at the time it was prepared. No other warranty, expressed or implied, is made as to the professional advice included in this Report.

It is prepared in accordance with the scope of work and for the purpose outlined in the contract 2229 dated 2017.

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Figures



Attachment 9.3.3







Appendix **B**

Certificates of Title

	FEIGL & NEWELL TITLE SEARCHERS		
	Broporty Boforonco		
	61 Mountain Highway, Wantirna		
Volume Folio	Registered Proprietors	Date	Status
V.11770 F.352	Crown Land Temporarily Reserved for Recreation Leisure And Tourism	04.12.2001	Current
V 8626 F 157	Transferred to Crown	30.11.2001	History
	Melbourne Parks & Waterways	13.03.1996	History
	Melbourne Metropolitan Board Of Works	27.09.1977	History
	The President Councillors And Ratepayers Of The Shire Of Knox	28.06.1966	History
	William Nest (Butcher)	12.07.1966	History
V.7220 F.886	William Nest (Butcher)	18.08.1949	History
	Elsie McLean	07.05.1947	History
V.6488 F.592	Frederick McDowall (Contractor)	03.10.1940	History
V.5751 F.143	John Sanders (Farmer)	02.12.1930	History
V.2747 F.250	John Sanders (Farmer)	06.04.1911	History
	Edward Ford (Butcher)	04.03.1903	History
	The Equity Trustees Executors and Agency Company Ltd	04.03.1903	History
	Allan Williamson (Market Gardener)	31.10.1899	History
V.9585 F.004	Transferred to Crown	30.11.2001	History
	Melbourne Parks & Waterways	13.03.1996	History
	Melbourne And Metropolitan Board Of Works	10.10.1984	History
V.4676 F.112	Country Roads Board	16.07.1974	History
	Frederick Finger (Orchardist)	31.08.1944	History
	Norman Taintor (Orchardist)	23.10.1936	History
	Archibald Sanders (Orchardist)	15.03.1923	History
V.5030 F.884	Transferred to Crown	30.11.2001	History
	Melbourne Parks & Waterways	13.03.1996	History
	Melbourne And Metropolitan Board Of Works	27.09.1977	History
	The President Councillors And Ratepayers Of The Shire Of Fern Tree Gully	03.09.1925	History





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ACTIVITY IN THE LAST 125 DAYS

NIL

DOCUMENT END

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Historical Search Copyright State of Victoria. This publication is copyright. No part may be reproduced by any process except in accordance with the provisions of the Copyright Act 1968 (Cth) and for the purposes of Section 32 of the Sale of Land Act 1962 (Vic) or pursuant to a written agreement. The information is only valid at the time and in the form obtained from the LANDATA REGD TM System. The State of Victoria accepts no responsibility for any subsequent release, publication or reproduction of the information. HISTORICAL SEARCH STATEMENT Land Use Victoria _____ Produced 31/10/2018 02:08 PM Volume 11770 Folio 352 Folio Creation: Created as crown land continued as computer folio Parent title Volume 11705 Folio 190 RECORD OF HISTORICAL DEALINGS Date Lodged for Date Recorded Dealing Imaged Dealing Type and Registration on Register Details 06/08/2016 06/08/2016 MI191394V N CROWN LAND STATUS REPORT RECORD OF VOTS DEALINGS Date Lodged for Date Recorded Dealing Imaged Registration on Register STATEMENT END HISTORICAL REPRINT(S) -----Historical Crown Parcel Report This is a report on the parcel from 1 October 2001. It is a statement of the historic legal status of the parcel.For historical research further documentation is available through Land Use Victoria. Land Description Allotment: 2202 Section: Township: Parish: Scoresby Standard Parcel Identifier (SPI): 2202\PP3478 End Of Land Description Details Status Details-Number of Status:1 Status: 1 of 1 Currency of Status: CURRENT Parcel Status: Crown land (reserved) Reserve Type: Temporary Reserve Purpose: CONSERVATION RECREATION LEISURE AND TOURISM Park: Administrator: Department of Environment Land Water and Planning Related instruments Gazette Year: Number: Page: Act Year: Number: Special Gazette Date: 04-DEC-2001 Page: 1 Number: 220 Order in Council Date: 04-DEC-2001 Reference Plan: Title Reference: Volume Folio: Status Remark: Standard Parcel Identifiers (SPI) that contain land excised from the parcel: None End Of Status Details

This SPI derived from Source SPI(s): 20B\PP3478

End Of Report

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Delivered by LANDATA®. Land Use Victoria timestamp 31/10/2018 13:08 Page 2 of 4 الم المستعلية في ÿ THE PRESIDENT COUNCILLORS AND RATPAYERS OF THE SHIRE OF KNOX is now the proprietor Registered 28th June 1966 FICE OF TIT R. L. No C525623 VICTORIA MELBOURNE AND METROPOLITAN BOARD OF WORKS is now the proprietor Registered 27th September 1977 LE OF No.G801502 1. 1. 6. I-line by PROPRIETOR MELBOURNE PARKS & WATERWAYS 378 COTHAM RD. KEW 3101 U1 27325 TRANSFER TO HER MAJESTY QUEEN-BLIZABETH II CANCELLED X918186R 30/11/01 T08626-157-1-0 1 18626 F.157



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Attachment 9.3.3

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Delivered by LANDATA®. Land Use Victoria timestamp 31/10/2018 13:08 Page 2 of 4 Vol. 2747 Fol. 549250 Transfer. Applie Red Ink 12 3170773. (734.0) 233) Application. RANSFER AS TO PART to albert Thomas Watkins registered on 34 the seconder 1930 numbered 1/ 38693. GANCELLED AS TO PART See Certificate of Title Vol 5751 For 1150144 L. Kennedy. area 1- 3-33 Associant Registrar of Titles Delivered on-line by LANDATA® 19 de November 1926 Probate of hor Will na been craned to Architald Sanders of Boronia Road Buyswook Cichardess IMAGED FOLIO - WARNING: TO BE USED FOR DIAGRAM, EASEMENT INFORMATION, DEPTH LIMITATIONS AND ANY DIAGRAM NOTATIONS The text of this Folio has been converted to a computer Folio and the effect of any dealing registered since the text conversion will appear on the computer Folio. Daked , 12 June 1937 S.P.C.L. Assistant Registrar of Titles TRANSFER AS TO PART I Lonald Ernes Finger on 114 Diamber 1985 -1718518 of Title -6261 1252008 A Finley Assessant Registrar of Titles TRANSFER AS TO PART IN Frederich George Malowall mentered on 1- October 1340numbered 1789571 CANCELLED AS TO FART Son Cartilicate of Title Vo6488 Fol1297592 Aug 22-20 Assessme Rothewison TRANSFER AS TO BALANCE 10 Geoffrey Archibald Landers resistored m 30th Warch 1950 mintered 22 9982 11 CANCELL Vol 7:392 1478278 99 15 Torden Allestant Registras of Title CANCELLED H. UNI THE



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VICTORIA

UNDER THE "TRANSFER OF LAND ACT

MELBOURNE AND METROPOLITAN BOARD OF WORKS is the proprietor of an estate in fee simple subject to the encumbrances notified hereunder in all that piece of land in the Parish of Scoresby County of Mornington being part of Crown-

Portion 20 which land is shown enclosed by continuous lines on the map - ---DATED the 10th day of October 1984 CE OF TITL ar of Titles (CTORIA ENCUMBRANCES REFERRED TO As to the land shown marked E-1 E DRAINAGE EASEMENT reserved by -

REGISTER BOOK

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Lotsearch Report

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Appendix D

Property Planning Report



PROPERTY DETAILS	
Address:	ł
Crown Description:	/
Standard Parcel Identifier (SPI):	2
Local Government Area (Council):	ł
Council Property Number:	I
Planning Scheme:	ł
Directory Reference:	I

61 MOUNTAIN HIGHWAY WANTIRNA 3152 Allot. 2202 PARISH OF SCORESBY 2202\PP3478 KNOX More than one - not listed. Knox Melway 63 C8

www.knox.vic.gov.au

planning-schemes.delwp.vic.gov.au/schemes/knox

UTILITIES

Rural Water Corporation:	Southern Rural Water
Melbourne Water Retailer:	Yarra Valley Water
Melbourne Water:	inside drainage boundary
Power Distributor:	UNITED ENERGY, AUSNET

STATE ELECTORATES Legislative Council: EASTERN METROPOLITAN Legislative Assembly: FERNTREE GULLY, FOREST HILL

Planning Zones

PUBLIC CONSERVATION AND RESOURCE ZONE (PCRZ) SCHEDULE TO THE PUBLIC CONSERVATION AND RESOURCE ZONE (KNOX) (PCRZ) PUBLIC PARK AND RECREATION ZONE (PPRZ) SCHEDULE TO THE PUBLIC PARK AND RECREATION ZONE (KNOX) (PPRZ) ROAD ZONE - CATEGORY 1 (RDZ1)





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Notwithstanding this disclaimer, a vendor may rely on the information in this report for the purpose of a statement that land is in a bushfire prone area as required by section 32C (b) of the Sale of Land 1962 (Vic).

PLANNING PROPERTY REPORT: 61 MOUNTAIN HIGHWAY WANTIRNA 3152

Page 1 of 8



Planning Overlays

ENVIRONMENTAL SIGNIFICANCE OVERLAY (ESO) ENVIRONMENTAL SIGNIFICANCE OVERLAY - SCHEDULE 2 (ESO2) (KNOX)





HO - Heritage

Note: due to overlaps, some overlays may not be visible, and some colours may not match those in the legend.

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PLANNING PROPERTY REPORT: 61 MOUNTAIN HIGHWAY WANTIRNA 3152



Planning Overlays

LAND SUBJECT TO INUNDATION OVERLAY (LSIO) LAND SUBJECT TO INUNDATION OVERLAY SCHEDULE (LSIO) (KNOX)



LSIO - Land Subject to Inundation

LSIO - Land Subject to Inundation Note: due to overlaps, some overlays may not be visible, and some colours may not match those in the legend.

SPECIAL BUILDING OVERLAY (SBO)

SPECIAL BUILDING OVERLAY SCHEDULE (SBO) (KNOX)



SBO - Special Building

Note: due to overlaps, some overlays may not be visible, and some colours may not match those in the legend.

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Planning Overlays

VEGETATION PROTECTION OVERLAY (VPO) VEGETATION PROTECTION OVERLAY - SCHEDULE 2 (VPO2) (KNOX)



VPO - Vegetation Protection Note: due to overlaps, some overlays may not be visible, and some colours may not match those in the legend.

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Planning Overlays

OTHER OVERLAYS

Other overlays in the vicinity not directly affecting this land DESIGN AND DEVELOPMENT OVERLAY (DDO) DEVELOPMENT PLAN OVERLAY (DPO) ENVIRONMENTAL AUDIT OVERLAY (EAO) PUBLIC ACQUISITION OVERLAY (PAO) SIGNIFICANT LANDSCAPE OVERLAY (SLO)



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Areas of Aboriginal Cultural Heritage Sensitivity

All or part of this property is an 'area of cultural heritage sensitivity'.

'Areas of cultural heritage sensitivity' are defined under the Aboriginal Heritage Regulations 2018, and include registered Aboriginal cultural heritage places and land form types that are generally regarded as more likely to contain Aboriginal cultural heritage.

Under the Aboriginal Heritage Regulations 2018, 'areas of cultural heritage sensitivity' are one part of a two part trigger which require a 'cultural heritage management plan' be prepared where a listed 'high impact activity' is proposed.

If a significant land use change is proposed (for example, a subdivision into 3 or more lots), a cultural heritage management plan may be triggered. One or two dwellings, works ancillary to a dwelling, services to a dwelling, alteration of buildings and minor works are examples of works exempt from this requirement.

Under the Aboriginal Heritage Act 2006, where a cultural heritage management plan is required, planning permits, licences and work authorities cannot be issued unless the cultural heritage management plan has been approved for the activity.

For further information about whether a Cultural Heritage Management Plan is required go to

http://www.aav.nrms.net.au/aavQuestion1.aspx

More information, including links to both the Aboriginal Heritage Act 2006 and the Aboriginal Heritage Regulations 2018, can also be found here - https://www.vic.gov.au/aboriginalvictoria/heritage/planning-and-heritage-management-processes.html



Aboriginal Heritage

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Further Planning Information

Planning scheme data last updated on 25 July 2019.

A planning scheme sets out policies and requirements for the use, development and protection of land. This report provides information about the zone and overlay provisions that apply to the selected land. Information about the State and local policy, particular, general and operational provisions of the local planning scheme that may affect the use of this land can be obtained by contacting the local council or by visiting <u>https://www.planning.vic.gov.au</u>

This report is NOT a Planning Certificate issued pursuant to Section 199 of the Planning and Environment Act 1987. It does not include information about exhibited planning scheme amendments, or zonings that may abut the land. To obtain a Planning Certificate go to Titles and Property Certificates at Landata - <u>https://www.landata.vic.gov.au</u>

For details of surrounding properties, use this service to get the Reports for properties of interest.

To view planning zones, overlay and heritage information in an interactive format visit http://mapshare.maps.vic.gov.au/vicplan

For other information about planning in Victoria visit https://www.planning.vic.gov.au

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Designated Bushfire Prone Area

This property is in a designated bushfire prone area. Special bushfire construction requirements apply. Planning provisions may apply.



Designated Bushfire Prone Area

Designated bushfire prone areas as determined by the Minister for Planning are in effect from 8 September 2011 and amended from time to time.

The Building Regulations 2018 through application of the Building Code of Australia, apply bushfire protection standards for building works in designated bushfire prone areas.

Designated bushfire prone areas maps can be viewed on VicPlan at http://mapshare.maps.vic.gov.au/vicplan or at the relevant local council.

Note: prior to 8 September 2011, the whole of Victoria was designated as bushfire prone area for the purposes of the building control system.

Further information about the building control system and building in bushfire prone areas can be found on the Victorian Building Authority website <u>www.vba.vic.gov.au</u>

 $Copies \ of \ the \ Building \ Act \ and \ Building \ Regulations \ are \ available \ from \ \underline{www.legislation.vic.gov.au}$

For Planning Scheme Provisions in bushfire areas visit <u>https://www.planning.vic.gov.au</u>

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DRAFT



Groundwater Resource Report

Groundwater Resource Report

Groundwater catchment: East Port Phillip Bay Depth to water table: < 5m Water table salinity (mg/	VICGRID94 Easting: 2 L): 1001 - 3500	518275 Northing: 2404907
Groundwater layers (Aquifers and Aquitards)	Depth below surface (m)	Groundwater salinity (mg/L)
QA Quaternary Aquifer sand, gravels, clay, silts	0 - 7	1001 - 3500
BSE Mesozoic and Palaeozoic Bedrock (basement) sedimentary (fractured rock): Sandstone, siltstone, mudstone, shale. Igneous (fractured rock): includes volcanics, granites, granodiorites.	7 - 207	1001 - 3500

There are no GMUs at this location

For further information about this report contact:

Department of Environment, Land, Water & Planning Email: ground.water@delwp.vic.gov.au

For further information on groundwater licensing in this area contact:

Southern Rural Water Corporation Phone: 1300 139 510 Email: srw@srw.com.au Website: www.srw.com.au

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Printed: 22 July 2019 Date Updated: 11 January 2019



Environment, Land, Water and Planning

How to read this report

Introduction

Groundwater is part of the water cycle. When rain or snow falls on land, some of it evaporates, some flows to streams and rivers, and some seeps into the soil. Some of the water in the soil is used by plants but some continues to move down through the soil and rock until all the pores and cracks are full of water. This is known as the water table and this water is called groundwater.

Groundwater is a finite resource that, like surface water, is allocated under the Water Act (1989). A Bore Construction Licence is required to drill for groundwater including for domestic and stock purposes. Taking and using groundwater for commercial or irrigation purposes requires an additional licence.

Purpose of this report

This report has been prepared to provide potential groundwater users with basic information about groundwater beneath their property. This includes the different geological layers, the depths of the layers and the salinity of groundwater in the layers. Information on the groundwater management units (GMU) and any associated caps on the volume that can be licensed (the PCV) are also provided. **Definitions and context**

Term Description Groundwater Catchment An identified area of the State within which groundwater resources are connected. Easting / Northing The VICGRID 94 coordinates of the spot that was selected on the interactive map. Groundwater Salinity Indicates the possible concentration of salts within the groundwater. The salt content indicates the possible uses of the water (see the Beneficial Use Table below). Fertilisers and other contaminants can also enter groundwater and affect its use. It is up to you to make sure that the groundwater you use is suitable for your purpose. An aquifer is a layer of soil or rock which stores usable volumes of groundwater. Aquifers are generally Aquifer limestones, gravels and sands, as well as some fractured rocks where the cracks in the rock are open and connected (some basalts, sandstones and limestones). How much water can be pumped from an aquifer depends on how much water is stored in pores and cracks, how well connected the pores and cracks are, and how thick the layer is. It is more likely that volumes of water for irrigation and urban water supply will come from gravels, sands, limestones and basalts that are at least 30 metres thick. Low volumes of water for domestic and stock use are likely from any aquifer greater than 10 metres thick. The advice above is a guide only, as the amount of water available can be highly variable. Actual pumping volumes can only be determined from drilling, appropriate construction and testing of a bore An aquitard is a layer of rock or soil that does not allow water to move through it easily, limiting its Aquitard capacity to supply water. Aquitards are generally silts, clays and fractured rocks (where there are few cracks in the rock or the cracks are poorly connected). Groundwater Management A collective term for groundwater management areas (GMAs) and water supply protection areas Unit (GMU) (WSPAs). GMAs and WSPAs are defined areas and depths below the surface where rules for groundwater use may apply. WSPAs often have caps on groundwater use and plans describing how the resource is managed. GMAs usually have caps on groundwater use and may have local plans and rules. All other areas are managed directly through the Water Act (1989). Always check with your local Rural Water Corporation to be sure that the information on the GMU is correct for your specific location. Permissible Consumptive A cap that is set under the Water Act (1989) declaring the total volume of groundwater that may be taken Volume (PCV) from the area. Once the PCV is reached, no additional extraction can be licensed for use within the area unless traded from another groundwater licence holder. Depth to Water Table This is an indication of the depth at which groundwater might first be encountered when drilling a bore. The depth can vary from year to year, and from place to place and may vary significantly from that indicated in this report.

Beneficial Use Table

Salinity range	Beneficial use as described by State Environment Protection Policy (Groundwaters of Victoria) s160								
(mg/L TDS)	Potable water - preferred	Potable water - acceptable	Potable mineral water	Irrigation	Stock water	Industry	Ecosystem protection	Buildings and structures	
<500	~	~	~	~	~	~	~	~	
501-1000		~	~	~	~	~	~	~	
1001-3500			~	~	~	~	~	~	
3501-13000					~	~	~	~	
13001+						~	~	~	

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Appendix F

Knox City Council Historic Development Plans











INSIDE EDGE SPORT AND LEISURE PLANNING

PROPOSED EASTERN FOOTBALL HUB

WANTIRNA SOUTH RECREATION RESERVE

50 MOUNTAIN HIGHWAY

WANTIRNA

Report No: 115706

24 March 2014 Date:

GEOTECHNICAL INVESTIGATION (PRELIMINARY)

By

A.S. JAMES PTY LIMITED 15 Libbett Avenue, Clayton South Vic. 3169 Tel: 613 9547 4811 Fax: 613 9547 5393

E-mail: melb@asjames.com.au

THIS REPORT SHALL ONLY BE REPRODUCED IN FULL

X:James\Geotechnical\Commercial\115706 Eastern Football Hub, WANTIRNA\FINAL REPORT DOCUMENTS\115706 Proposed Eastern Football Hub, docx



15 Libbett Avenue Clayton South Vic 3169 Tel: (613) 9547 4811 Fax: (613) 9547 5393 E-mail: <u>melb@asjames.com.au</u>

1. INTRODUCTION

- 1.01 <u>Investigation Requested By</u>: The geotechnical investigation was commissioned by Mr Michael Bodman Bolton of Inside Edge Sport and Leisure Planning via email correspondence dated 21st February 2014.
- 1.02 <u>Purpose of Investigation</u>: The scope for this investigation is as follows:

General: It was proposed to develop the Wantirna South Recreation Reserve into an Australian Rules Football training and administration hub. In doing so, bringing together training and development functions of the Eastern Football League, Eastern Rangers TAC, Eastern Region Umpires, AFL Victoria and Hawthorn Football Club.

Proposed Infrastructure: The proposed developments to the site are as follows:

- The conversion of the existing playing field (eastern oval) from grass to an AFL/CA approved synthetic oval
- The development of disused open space (current picnic/play area) to premier quality natural turf, sand based oval of MCG size and quality
- The development of a 1500m2 (double story) office administration, social and change room complex
- The development of roadways and car parking to meet the demands of the upgraded site

Information provided indicates the site was believed to be a landfill pre 1968 and the existing road ways were developed in 1998. The likely specifications for the proposed main natural grass playing field or (Western Oval) are in accordance with the '*Colac Central Reserve*' document provided. Specifications for the proposed synthetic oval (Eastern Oval) are understood to be in accordance with the '*Australian Football League and Cricket Australia Handbook of Testing for Synthetic Turf* also provided by the client.

(PRELIMINARY) Geotechnical Investigation 24 March 2014 <u>Proposed Eastern Football Hub, Wantirna South Recreation Reserve, 50 Mountain Hwy WANTIRNA REF:</u> 115706

Discussions: The following are requested aspects of the site to be discussed.

- Key issues associated with the sub-surface conditions and how they may impact the proposed infrastructure
- Earthworks
- 1.03 <u>Geology:</u> At least part of the site, as previously indicated, was historically used as a landfill before being converted into parklands and recreational space. The Geological Survey of Victoria, 1:63,360 Series, Ringwood Sheet, indicates the subject site to be underlain by sedimentary deposits which are of the Silurian age and form part of the Dargile Formation. Typically, these deposits comprise of laminated and current bedded sandstones, interbedded with massive siltstones and shales. Some alluvial deposits may exist in the lower elevations.
- **1.04** <u>Field Methods:</u> As part of the geotechnical investigation the following field methods were incorporated:
 - Auger Drilling: All boreholes were drilled using a truck mounted Gemco HP 7 rotary drilling rig equipped with continuous flight 125 millimetre diameter augers fitted with tungsten carbide drill bits.
 - ii) In-situ Vane Shear Strength Testing: In-situ vane shear strength testing was carried out within the cohesive soils at shallow depths using a Pilcon hand vane tester. The tests were conducted in accordance with the test procedure outlined in Australian Standard 1289, "Methods of Testing Soils for Engineering Purposes". Test Method F2. 1, June 1977.
 - iii) Standard Penetration Testing: Standard penetration testing was conducted at regular intervals within the boreholes in accordance with the test procedure outlined in Australian Standard 1289, "Methods of Testing Soils For Engineering Purposes," Test Method 6.3.1, June 1993.in accordance with Australian Standard AS 1726 1993, "Geotechnical Site Investigations."
 - iv) Logging of Soil Profiles: The soil profile encountered in the borehole was logged in accordance with Australian Standard AS 1726 1993, "Geotechnical Site Investigations."

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1.05 <u>Laboratory Test Methods:</u> All soil samples were transferred to A.S. James' National Association of Testing Authorities (NATA) registered Clayton South laboratory, where mechanical testing was undertaken by a team of trained laboratory technicians. All laboratory testing was performed in strict accordance with the test methods outlined in Australian Standard AS 1289, "Method of Testing Soils for Engineering Purposes," as follows:

Suggested testing in the brief was modified too.

• Atterberg Limits

AS 1289 Test Method 3.1.1, 3.2.1, 3.3.1.3.4.1

2. RESULTS

2.1 FIELD TESTING

2.1.1 <u>Bore Locations</u>: Twenty (20) boreholes were drilled at the locations indicated on Figures 1 and the logs of these boreholes are given on Figures 2 – 17. The results of standard penetration tests are given on the logs.

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2.1.3 <u>Sub-Surface Soil Profile:</u> The results of the borehole drilling program conducted at the site, have been summarised in Table 1 & 2 below.

Borehole	Depth Range of crushed rock (m) (FILL)	Depth Range of Filling (Sandy silt) (m) (FILL)	Depth Range of Silt/ Clay Cap (m) (FILL)	Depth Range of Historic Tip (m) (FILL)	Depth Range of Reworked Natural Material (m) (FILL)	Depth Range of Natural Clayey Silt (m) (SILT) (MH)	Dargile Formation Depth Range of Natural Silty Clay (m) (CLAY) CH /CL
1	-	0-0.2	0.2-0.6	0.6-4.0	-	-	4.0-4.5
2	-	0-0.6	-	0.6-2.0	-	- /	
3	-	0-0.2	-	0.2-3.0	-		
4	-	0-0.1	-	0.1-2.0	-	<u> </u>	-
5	0-0.1	-	-	0.1-2.0	-		-
6	0-0.1	-	0.1-0.6	0.6-2.0	-	-	-
7	-	-	0-2.2	2.2-3.0	X · \		-
8	0-0.15	-	-	-	0.15-1.7	-	1.7-3.0
9	0-0.1	-	-	-	0.1-3.0	-	3.0-3.4
10	0-0.05	-	0.05-1.0	1.0-1.4	-	1.4-1.6	1.6-6.4
11	0-0.05		0.05-1.6	1.6-3.5	<u> </u>	-	3.5-6.4
12	0-0.05	-	0.05-1.7	1.7-4.0	y -	-	4.0-6.4
13	0-0.05	-	$\langle \mathcal{N} \rangle$	- - -	0.05-2.5	-	2.5-6.0
14	-	-	1.4-1.8	1.8-4.5	0-1.4	-	4.5-4.6

Table 1: Summary of Sub-surface Soil Profiles within Depths Investigated (Bores 1-14).

Borehole	Depth Range of Filling (Silty Sand) and (Sand / Silt / Gravel) (m) (FILL) Playing Surface	Depth Range of Reworked Natural Material (m) (FILL)	Depth Range of Natural Silt (m) (SILT) (MH)	Dargile Formation Depth Range of Natural Silty Clay or Silty Sand (m) (CLAY) CH /CL, (SAND) (SM)
15	0-0.3	-	-	0.3-2.0
16	0-0.3	0.3-1.0	-	0.3-1.0
17	0-0.3	-	-	0.3-2.0
18	0-0.3	0.3-1.3	-	1.3-2.0
19	0-0.1	-	0.1-0.3	0.3-2.0
20	0-0.3	0.3-1.0	-	0.3-1.0

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Table 1: Summary of Sub-surface Soil Profiles within Depths Investigated (Existing Sporting Oval).

Bores 1-4 were carried out at the picnic / play area of the site, bores 5, 6, 7, 8, 9 & 14 were carried out in the car-park area of the site and bores 10-13 were carried out within the proposed new double story building envelope. Bores 15-20 were carried out on the existing sports oval.

A summary of each of the soil units encountered at the subject site is provided as follows:

- <u>Crushed Rock Fill:</u> *Car Park Areas:* Within the car parking areas, a layer of dry, medium dense silty and sandy crushed rock fill of thickness ranging from 0.05-0.15m generally was encountered. The suitability for re-use around the site or as future sub-base has not yet been determined.
- <u>Sandy Silt: *Picnic / Play Area*</u>: A layer of generally brown, dry, medium dense sandy silt with an organic content was encountered within the picnic / play area of the site, ranging in depths of 0.1-0.6 m.
- <u>Sand:</u> 'Capping' Fill used to develop the playing surface of the existing sports oval: A thin layer of moist brown silty sand in medium dense condition was encountered approximately 0.1 m in depth over the existing sports oval to the east. Underlying this a layer of brown sand / silt / gravels was encountered of medium density, 0.2 m thick.
- <u>Silt / Clay Capping</u>: Predominantly within the existing car parking areas and accessible locations of the proposed double story building envelope. A likely reworked natural material was encountered, mostly a yellow brown / orange brown occasionally mottled red silty clay however

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appearing as a clayey silt at times. This layer was typically of stiff consistency or medium dense, dry tending moist with a gravel content.

- <u>Historic Landfill:</u> Underlying much of the site in particular the proposed western natural grass oval is the historic tip. This unit typically presented as a grey brown / brown grey moist silty clay of medium density. Accessory materials encountered includes, glass, brick, plastic, metal, gravels and other miscellaneous items such as the sole of a shoe encountered in BH14. Within the depths investigated this unit ranged from 0.1 m up to 4.0 m below ground surface. Typically over the picnic / play area the sandy silt was overlying the historic land fill and within the car park and proposed building envelope where encountered, the historic landfill was underlying the silt / clay capping. Perched water tables were encountered at BH1 and BH14 at approximately 3.0 m above the underlying clay.
- <u>Reworked Natural Material:</u> Encountered at boreholes in close proximity to the existing sporting oval and eastern parts of the sporting oval. Reworked natural materials were encountered of characteristics to those similar to the natural undisturbed materials underlying the site in addition to what appears to be a crushed siltstone fill. This profile generally appeared as a dry to moist yellow brown / orange brown / grey occasionally mottled red stiff silty clay, in medium dense condition.
- <u>Natural Clayey Silt (MH)</u>: Observed in BH10 below the historic tip profile a thin layer of pale grey moist, loose to medium dense clayey silt was encountered. It is possible this was the original ground surface level before filling associated with the historic tip began.
- <u>Natural Silurian Age Dargile Formation:</u> Encountered underlying historic tip fill, natural clayey silts or reworked natural materials, a grey / orange brown stiff silty clay in moist condition was encountered. At deeper depths greater than 5.0 m sub-angular ironstones were encountered including variably ferruginous occasional grit and gravel bands. Of note, the silty clay tended to a sandy / silty clay at approximately 5.0 m within BH11.
- **2.1.5** Discussion of possible historic earthworks: With reference to the attached Cross Section A-A', the following is observed site notes regarding the possible previous earthworks at the site. The bore logs indicate that a possible earthworks program previously conducted at the site included the cutting of natural materials from the eastern areas of the existing oval and subsequent benching by filling of the remaining areas, westwards over the oval. In addition, placement of natural materials over the historic landfill, predominantly over the car park areas and existing

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building area. It appears the materials were also placed in a somewhat uncontrolled manner. The historic landfill appears to reduce in depth as it approaches the existing oval however, relatively significant depths of landfill were still recorded west of the existing pavilion adjacent to the sports oval.

2.1.6 Ground Water: No permanent free ground water was encountered at the time of the site investigation and none would normally be expected within the depths investigated. However at two locations a perched water table was encountered within the historic landfill profile at approximately 3.0 m. It should be noted, however, that following prolonged periods of rainfall the surface fills, historic landfill and reworked natural materials, clayey silt and shallow silty clay may be susceptible to moisture ingress, thereby significantly reducing the workability and strength of both the surface fill and the underlying sands at shallow depths.

2.2 LABORATORY TESTING

- 2.2.1 <u>Test Program:</u> Upon receipt in the laboratory, the disturbed soil samples retrieved from Boreholes were prepared as necessary before Atterberg limit testing was carried out.
- 2.2.2 <u>Test Results:</u> The results of the laboratory Atterberg limits are given attached.

3. DISCUSSION & RECOMMENDATIONS

- 3.1 PROPOSED STRUCTURE: Double storey, well-articulated and able to accommodate minor movement.
- **3.1.1** <u>Site Classification:</u> In selecting an appropriate foundation arrangement the following factors have been taken into account:
 - Sub-surface soil profiles and results of visual site observations
 - Local geological conditions;
 - Knowledge of the site and previous likely earthworks on this development.

If required the site should be classified as Class 'P' in accordance with AS 2870 – 2011 and fully suspended construction will be necessary.

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- **3.1.2** <u>Piled Footings:</u> Pile footings should be adopted. Considering the variation of historic landfill depths, the piles need to extend into the silty clay below any clayey silt, reworked natural material or historic fill.
- **3.1.3 Bored Piers:** Bored piers could be considered although some instability may be encountered and temporary liners will be necessary.

If required for a preliminary design a bearing pressure of 300 kPa could be adopted subject to a penetration of 1.0 m into the underlying clay. A skin friction of 30 kPa could be adopted below this level.

3.1.4 Provided that adequate protection is provided against the possibly aggressive ground conditions that may prevail with the historic fill profile, the use of precast concrete piles appears appropriate for the proposed development.

<u>Pile Load Capacity:</u> Provided that adequate protection is provided against the possibly aggressive ground conditions that may prevail within the historic fill profile, the use of precast concrete piles appears appropriate for the proposed development.

Considering the potential for variation of the subsurface profile across the site the piles will need to be driven into the "Dargile Formation" layer and piles need to be driven until the required set is achieved.

The load carrying capacity of driven piles in the stiff clay will significantly be shaft resistance. In this site, it is expected significant load development will not occur until up to in all likelihood 4.0 metres due to the historic landfill and possibly uncontrolled fill. Obviously, variability in both the depth of historic fill and the properties of the Dargile Formation across the site will influence final piling depths and embedment depth variation will exist.

As per our calculations, based on a conservative depth of fill at 4.0m a 350 sq. smooth pile driven 3.0 metres into the Dargile Formation will provide a working load of 160 kN. That is a total pile depth of approximately 7.0m, although site variation and density will determine ultimate depths, and increased pile sizes could be considered. Greater bearing pressures can be provided subject to further geotechnical investigation to deeper depths within the 2-story building envelope until rock is encountered, this is estimated in the order of 8-10 m below ground surface level.

Piling contractors should make their own assessment of piling conditions and load carrying capacities of proprietary pile types. Design geotechnical strength is the calculated design ultimate geotechnical strength multiplied by a geotechnical strength reduction factor. As per the piling code (Revision of AS 2159-1995) the geotechnical strength reduction factor should be determined considering the site, design and installation of piles.

We emphasise that the historic landfill depths are variable up to 4.0 m recorded within the building envelope and variable depths can be expected and will occur.

- **3.1.5** The use of CFA (Continuous Flight Auger) or driven piles could be considered and the logs should be referred to proprietary piling contractors.
- **3.1.6** <u>Pile Settlements:</u> Pile settlements could be assessed using a Young's Moduli (E) for the stiff silty tending sandy clay of 50 MPa.
- **3.1.7** <u>Pile Groups:</u> The total working load for the pile will be developed by a combination of end bearing and shaft friction into the silty tending sandy clay.

It is recommended that the following precautionary measures be taken during the pile driving operations.

- Continuous monitoring of pile heads during installation to ensure eccentricities do not develop.
- A construction sequence be developed to minimise potential interaction (suggest three diameters) of piles and drive central, inner and outer.
- Pre-bore (with casing, if necessary) to remove material and obstructions in the historic landfill.
- Ensure the use of a proprietary tensile capacity joint or during monitoring (both laterally and vertically) have the capacity to 'reset' piles lifting.

It is recommended the driven piles be initially checked using dynamic pile testing (Pile Driving Analyser) and confirmed using CAPWAP analysis or similar on selected piles (10% in Number, but subject to Quality Risk Factors).

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3.1.8 <u>Pile Downdrag</u>: The down-drag stress at any point on the pile within the historic landfill could be calculated using the following equation:

	$f_{nf}=0.3\sigma'_{v}$
where f _{nf}	is the downdrag stress
σ'_{v}	is the effective overburden stress (adopt a bulk unit weight
	of 15 kN/m ³ for the fill including capping, reworked natural
	materials and historic landfill)

Pile down drag should be considered to a depth 1.0m above the base of the stiff clay. Based on the information this can be considered at approximately 3.0 m in depth.

- **3.1.9 Protection of Piles:** The possibility remains of the fill to be of aggressive nature to concrete, in all likelihood it will be necessary to provide an appropriate level of protection for the proposed precast concrete piles. Such protection should include the use of cement rich, high quality, very dense, impervious concrete, which should be placed with a high degree of vibration. As a guide the concrete should contain not less than 425 kg/m3 cement and 50 kg/m3 flyash placed with a super-plasticiser at a water cement ratio of 0.30. Care will also need to be taken to ensure that the steel segmented pile joints are not located within the uppermost 4.0 m. Other proprietary options could also be considered.
- 3.1.10 <u>Earthquake Loading</u>: In accordance with Australian Standard 1170.4-2007, Part 4, "Earthquake Actions in Australia", site sub-soil class of C_e shallow soil site and Hazard Factor (Z) of 0.09 should be adopted for the design of the proposed structure at the subject site.
- 3.1.11 Discussion of Settlements of proposed premier quality natural turf, sand based oval (Proposed Western Oval): In accordance with the provided likely specifications for the proposed premier quality natural turf sand based oval taken from the Colac Central Reserve, it is stated within Section 8.1 of the document that the 'soil rootzone layer' will be to a consolidated depth of 250 mm so that the finished surface has a tolerance of + or 10 mm.'
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Given the ongoing settlement that will occur underlying the site and the relatively poor engineering quality of the landfill materials, it is envisioned that due to these inevitable settlements / differential settlements that will take place at the proposed location, the tolerance of + or - 10mm will be significantly exceeded. Drainage in particular is likely to be affected due to the differential settlement of the oval.

Significant cost will be expended in remediating this area of the site and in fact would likely be cost prohibitive. A 'use and maintain' approach would appear all that is reasonable.

3.1.12 Discussion of Settlements of Australian Football League and Cricket Australia Grade Synthetic Turf Oval (Eastern Oval): After reviewing the provided handbook, regarding the specifications for a synthetic oval. Settlement tolerances were not provided, however it is assumed that this type of construction, similar to the natural turf, sand based oval is sensitive to movements and drainage is critical.

As the synthetic turf oval is proposed to be located at the existing sports oval. Given the presence of reworked natural fill materials, particularly from the centre of the existing sporting oval heading westwards. The likely resulting consolidation settlements are expected to be outside + or -10 mm, if the proposed synthetic turf oval was to be located at the existing ovals location. The use of an impact roller on this existing fill may be sufficient to reduce the settlements in the fill to acceptable limits. However, more and intrusive investigation is required with test pits.

3.1.13 <u>Discussion of Environmental Condition of Site:</u> Given the site history as to a landfill. It is understood there are no available records to the type and quantity of material that was bought on site. Due to the uncontrolled nature of the filling, it is possible and likely that the soil is contaminated to some degree, however to what degree at this stage is largely unknown and until firm proposals exist no testing is warranted.

Landfills can be typically heterogeneous in both physical and chemical composition. As such, environmental preliminary sampling and analytical testing can be misleading as both the physical and chemical composition can change over relatively short distances. Therefore, more detailed environmental investigations are recommended as follows; however, this is outside the scope of this investigation.

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A preliminary phase 1 environmental assessment focuses on site history in the aim of determining possible environmental impacts by ascertaining possible contaminants of concern related to the sites previous use.

Upon the completion of a site history study, a limited sampling and analysis program can be completed testing for contaminants of concern using a specific sampling strategy. Collection of ecological information on background inorganic element and organic compounds present would also be completed. In doing so, this gathered information would aid in reviewing results and determining if possible impacts may be imposed on ecological receptors. Contaminant threshold concentration levels can be adopted for human health and ecological receptors in accordance with the current '*National Environmental Protection (Assessment of Site Contamination) Amendment Measure 2013 (No.1)*.'

A preliminary off-site disposal categorisation can be provided in accordance with Victorian EPA Publication IWRG-621-June 2009 'Soil Hazard Categorisation and Management'. Along with any necessary details and recommended practices for disposing the soil off-site. It should be noted as above, given the likely heterogeneous nature of landfill material a conservative approach is likely to be adopted.

The production of landfill gases will also require investigation at the social and change room complex.

Note: Given the high content of municipal/domestic waste and industrial wastes in the historic landfill profile, the fill cannot be used as engineered / structural fill. A prescribed industrial waste category will need to be assigned to the landfill material and if required disposed of at a licenced facility.

Upon the completion of the preliminary Phase 1 environmental assessment, a further phase 2 investigation may be recommended to further explore / delineate any contamination found or determine an appropriate remediation strategy in line with a hierarchy of control.

In the case construction was to commence at the site, a site specific environmental management plan should be completed addressing any contamination on site to inform any contractors of the conditions, and how to manage them for both safety, and compliance. (PRELIMINARY) Geotechnical Investigation 24 March 2014 Proposed Eastern Football Hub, Wantirna South Recreation Reserve, 50 Mountain Hwy WANTIRNA REF: 115706

3.2 PAVEMENT CONSTRUCTION & GROUND FLOOR SLAB

3.2.1 <u>Pavements:</u> All building pavements should be fully suspended.

3.2.2 External Pavements Constructed on Fill: Some ongoing settlements within the fill underlying the subject site will result in distortion and maintenance requirements to pavements constructed on the existing ground surface level. Recognising this, any proposed pavements should be constructed on an adequately prepared subgrade with maximum possible grades to minimise future maintenance and ponding of water. Settlements are difficult to assess however may be as much as 50mm. (Except for the landfill area)

Flexible pavements and floor slabs constructed on an adequately prepared fill subgrade may be designed using a Design CBR value of 2.0 % Rigid pavements, on the other hand, constructed on a similar subgrade, may be designed using a Modulus of Subgrade reaction value of 20 kPa/mm.

Alternatively, rigid pavements may be designed in accordance with the Cement and Concrete Association of Australia, 1997 publication, "Industrial Pavements - Guidelines for Design, Construction and Specification" using long and short term Young's Moduli of 10 and 12 MPa respectively.

The effects of movements on any proposed rigid pavements can be minimised by incorporation of positive load transfer devices such as dowels.

It should be pointed out, however, that the pavement design parameters recommended above are given subject to the subgrade preparation outlined in Clause 3.2.2 and 3.2.3 being carried out, in addition to adequate subgrade drainage control, as outlined in Clause 3.2.4.

3.2.3 Preparation Fill Subgrade: The fill subgrade should be compacted with a heavy weight vibrating sheepsfoot roller. A minimum dry density ratio of 98% of the maximum dry density value determined by the Standard Compaction Test in accordance with Australian Standard AS1289 5.1.1 - 1993 should be achieved.

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Under extreme conditions, if the subgrade exists in a saturated state, it may be necessary to strip the saturated clays and replace these with suitable granular fill compacted to a dry density not less than 98% of the maximum dry density value determined by the Standard compaction test in accordance with AS1289 5.1.1.

It should be appreciated that the long term performance of the proposed pavements constructed on a fill subgrade significantly depends on the subgrade moisture conditions at the time of construction.

If very wet conditions are prevalent at the time of construction then there is the risk of some subsequent shrinkage occurring as the clay dries out. On the other extreme, if very dry conditions have prevailed for a significant period then there could be a risk of some resulting heave as the clays wet up.

As outlined above, impact rolling could be considered to improve performance.

- **3.2.4** <u>Long Term Subgrade Moisture Control:</u> It is considered essential for the long term performance of the proposed pavements at the subject site that both an effective surface and lateral cut-off drainage system be provided and maintained to minimise the risk of moisture migration into both the pavement sub-base and subgrade layers. Under no circumstances should the pavement and subgrade layers be permitted to remain in a saturated condition.
- **3.2.5** <u>Earthworks:</u> It is pointed out that the clay/silt fills including the capping layer over much of the site are notoriously difficult to work and if not compacted at or very close to the optimum moisture content, can exhibit measurable volume change with time. As such, the use of these clays will require close supervision.

Any imported structural fill should essentially be of a granular nature. All fill material should have a nominal particle size of 75 millimetres or less and, if required, a guide for selecting an appropriate material would be as follows:

• Plasticity Index. x Percentage Passing 0.425 millimetres (AS Sieve) less than or equal to 600

Structural fill should be compacted in layers not greater than 200 millimetres when loose and should be compacted to a dry density not less than 98% of the maximum dry density value determined by the Standard Compaction Test in accordance with Australian Standard AS 1289 5.1.1 - 1993 using an appropriate heavy weight vibrating roller.

During compaction, the fill material should have moisture content within the range 85% to 115% of the optimum moisture content as determined by the Standard Compaction Test in accordance with AS 1289 E1.1 - 1993.

3.2.6 <u>Fully Suspended Construction:</u> If the anticipated pavement settlements cannot be tolerated it will be necessary to fully suspend the proposed pavements on a series of piles.

3.3 GENERAL

- **3.3.1** <u>Underground Services:</u> Ongoing settlements within the significant depths of fill underlying the subject site will result in distortion and maintenance requirements to underground services at the subject site. Recognising this, the following precautions should be taken for any proposed underground services at the subject site:
 - All services should be laid with maximum possible grades to minimise future maintenance requirements.
 - Flexible joints should be incorporated for all services.
 - Services connecting to the proposed structure should be appropriately sleeved at the point of connection to allow for differential settlements where the structure is suspended.
- **3.3.2** <u>Inspection of Footing Excavations:</u> All footing excavations must be examined to ensure that the required founding soil has been exposed. Any unusual features must be reported to this office immediately in order to ensure that the recommendations outlined in this report remain relevant.
- **3.3.3** <u>General:</u> The Modulus of Subgrade reactions specified throughout the report are referred to as the K(0.3) value in most literature on the subject. As such, they are directly relevant where point loads are critical, but otherwise will require amendment depending on the value of the loading and geometry of the structural element involved. We would be pleased to advise further once relevant details are available.

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Conditions may change with the seasons. In particular, the surface silts and residual clays underlying the site at shallow depths may become saturated and unworkable following prolonged periods of rainfall.

The above recommendations are based on the bore and test results, together with experience of similar conditions and are expected to be typical of the area or areas being considered. Nevertheless, all excavations should be examined carefully and any unusual feature reported to us in order to determine whether any changes might be advisable.

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REVIEWED BY T.J. HOLT MIEAust CPEng EC-1022 A.S. JAMES PTY LTD J .T. HOLT BEng

Grad Inst of Engineers A.S. JAMES PTY LTD

X: James Geotechnical/Commercial/115706 Eastern Football Hub, WANTIRNA/FINAL REPORT DOCUMENTS/115706 Proposed Eastern Football Hub.docx



A	A.S.JAMES PTY. LTD.	Location: PROPOSED EASTERN Borehole: 1					orehole: 1
G	eotechnical Engineers		FOO	OTBALI	HUB, WANT	RNA	
		Job No.	11570)6 Porcho	d water table a	tann	Date: Mar '14
Soil Type	Description	Depth	aler.	Tests	Results	арри	oximately 5.0 m.
jpc							
FILL	Brown Silt	0					
	Accessory Materials: Trace sand and grass roots						
FILI	Yellow brown / orange brown stiff clay	0.2 .	+				
	Accessory Materials: Gravels, silt		///				
	Dry, medium dense		///				
	Likely Reworked Natural Material	0.6.					
	(Capping Layer)						
			///				
			$\langle / / \rangle$				
			///				
FILI	Brown grey / grey brown firm clay						
	Accessory Materials: Silt, plastic, brick fragments,		///				
	glass, gravels		///				
	Moist, medium dense						
			$\langle / / \rangle$				
			///				
			///				
			///				
			///				
			///				
CLAY	Grey / orange brown	4.0	///				
(CH/CL)	Silty						
	Moist, stiff						
	Dargie Formation						
	BOREHOLE TERMINATED	4.5					
		· ·					
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohes	ion	L.L. Liquid Lim	it	F ierer-
I Undistur	bed Sample - Diameter Stated	Ø Friction	Angle 		P.L. Plastic Lir	nit	rigure
s vane S	near orrengin Penetrometer Resistance	w Moistur	isity Conter	ht	LS Linear Sh	rinkago	<u> </u>

A	A.S.JAMES PTY. LTD.	Location: PROPOSED EASTERN Borehole: 2				
G	eotechnical Engineers	Job No 11	FOOTBALI	L HUB, WANTIRNA	Date: Mar '14	
		Ground Wate	er: NIL		Dute: Mar 14	
Soil Type	Description	Depth				
FILL	Grey brown silt Accessory Materials: Trace gravels Dry, medium dense	0 0.6 .				
FILL	Brown grey / yellow brown / grey firm clay Accessory Materials: Silt, plastic, gravels Dry to moist, medium dense Historic Landfill					
	BOREHOLE TERMINATED Note: This bore was terminated still within the historic landfill profile	2.0				
. Otrad	d Department Test. N blows (170 mm in an					
+ Standar I Undistu s Vane S p Pocket	a Penetration Test - N blows/150mm. incr. rbed Sample - Diameter Stated hear Strength Penetrometer Resistance	c Apparent Co Ø Friction Ang P Wet Density w Moisture Co	onesion gle y ontent	L.L. Liquid Limit P.L. Plastic Limit P.I. Plasticity Index L.S. Linear Shrinkage	Figure 3	

A	A.S.JAMES PTY. LTD.	Location:	PROPOSE	D EASTERN	Borehole: 3
G	eotechnical Engineers	1	FOOTBAL	L HUB, WANTIRNA	_
		Job No.	115706		Date: Mar '14
Soil Type	Description	Depth	Tests	Results	
FILL	Grey brown silt Accessory Materials: Gravels	0	777		
	Dry, medium dense				
FILL	Brown grey / orange brown / grey firm clay Accessory Materials: Silt, plastic, gravels, copper Dry to moist, medium dense Historic Landfill Note: Very Moist at the bottom of the borehole				
	BOREHOLE TERMINATED	3.0			
+ Standar I Undistur s Vane S p Pocket	d Penetration Test - N blows/150mm. incr. rbed Sample - Diameter Stated hear Strength Penetrometer Resistance	c Apparen Ø Friction P Wet Der w Moisture	t Cohesion Angle nsity e Content	L.L. Liquid Limit P.L. Plastic Limit P.I. Plasticity Index L.S. Linear Shrinkage	Figure 4

ļ	A.S.JAMES PTY. LTD.	Location: PROPOSED EASTERN Borehole: 4 & 5					
G	eotechnical Engineers		FOOTBALL	HUB, WANTIRNA			
		Job No.	115706		Date: Mar '14		
Soil Type	Description	Depth	Tests	Results			
FILL	Grey brown silt Accessory Materials: Gravels, grass roots Dry, medium dense	0 0.1 .					
FILL	Grey brown / orange brown / grey firm clay Accessory Materials: Silt, plastic, gravels, glass Dry to moist tending very moist, medium dense Historic Landfill						
BH5 FILL	Grey brown crushed rock	2.0					
	Accessory Materials: Silt, sand Dry, medium dense	0.1 .					
FILL	Grey brown / orange brown / grey stiff clay Accessory Materials: Silt, cloth, gravels, steel Dry to moist, medium dense Historic Landfill						
	BOREHOLE TERMINATED	2.0					
+ Standar	rd Penetration Test - N blows/150mm. incr.	c Apparent	Cohesion	L.L. Liquid Limit			
I Undistu	rbed Sample - Diameter Stated	Ø Friction	Angle	P.L. Plastic Limit	Figure		
s Vane S	Shear Strength	P Wet Den	Sity	P.I. Plasticity Index	5		

4	A.S.JAMES PTY. LTD.	Location:	PROPOSE	D EASTERN B	Borehole: 6
G	eotechnical Engineers		FOOTBALL	HUB, WANTIRNA	
		Job No.	115706		Date: Mar '14
Soil Type	Description	Depth	Tests	Results	
FILL	Grey brown crushed rock	0			
FILL	Accessory Materials: Silt, sand Dry, medium dense Yellow brown / orange brown mottled red	0.1 .			
	stiff clay Accessory Materials: Gravels, silt Drv. medium dense	 0.6 .			
	Likely Reworked Natural Material (Capping Layer)				
FILL	Grey tending dark grey firm clay Accessory Materials: Silt, plastic, glass, steel Moist, medium dense Historic Landfill				
	BOREHOLE TERMINATED	2.0			
		· · ·			
+ Standar	d Penetration Test - N blows/150mm incr	c Apparen	t Cohesion	L.L. Liquid Limit	
I Undistu	rbed Sample - Diameter Stated	Ø Friction	Angle	P.L. Plastic Limit	Figure
s Vane S	hear Strength	P Wet De	nsity	P.I. Plasticity Index	6
p Pocket	Penetrometer Resistance	w Moisture	e Content	L.S. Linear Shrinkage	

F	S.JAMES PTY. LTD.	MES PTY. LTD. Location: PROPOSED EASTERN Borehole: 7			
G	eotechnical Engineers		FOOTBAL	L HUB, WANTIRNA	
		Job No.	115706		Date: Mar '14
Soil Type	Description	Ground W	ater: NIL	Besults	
FILL	Yellow brown silt	0	7.7.7	riesuits	
	Accessory Materials: Silt, sand, tending clayey Dry to moist, medium dense Likely Reworked Natural Material (Capping Layer)				
		· · · · · · · · · · · · · · · · · · ·			
		· ·			
		2.2 .			
FILL	Dark grey brown firm clay Accessory Materials: Silty, plastic, glass, steel, brick fragments Moist, medium dense Historic Landfill	· · ·			
	BOREHOLE TERMINATED	3.0			
		· · · · · · · · · · · · · · · · · · ·			
					r
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparent	Cohesion	L.L. Liquid Limit	Figure
	Deu Sample - Diameter Stated		Angle	P.L. Plastic Limit	rigure 7
p Pocket	Penetrometer Resistance	w Moisture	Content	L.S. Linear Shrinkage	

A	A.S.JAMES PTY. LTD.	Location: PROPOSED EASTERN Borehole: 8			Borehole: 8	
G	eotechnical Engineers	FOOTBALL HUB, WANTIRNA			Α	
		Job No.	115706			Date: Mar '14
Soil Type	Description	Denth		ete	Results	
		Deptil		5010	Tesuis	
FILL	Accessory Materials: Silt, sand Dry, medium dense	0.15.				
	stiff clay / silt Accessory Materials: Gravels	· ·				
	Likely Reworked Natural Material	· · ·				
		· 				
FILL / CLAY (CH/CL)	Orange brown stiff clay stiff clay Accessory Materials: Silty	1.3 .				
	Dry to moist, medium dense Possibly Reworked Natural Material	1.7 .				
CLAY (CH/CL)	Grey / orange brown Silty Moist, stiff					
	depths including occasional grit and gravel bands, variably ferruginous Dargile Formation					
	BOREHOLE TERMINATED	3.0				
		· ·				
		· ·				
		·				
		· .				
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohesion	L	L. Liquid Limit	
I Undistur	bed Sample - Diameter Stated	Ø Friction	Angle	F	P.L. Plastic Limit	Figure
s Vane S	near Strength	P Wet Der	nsity	F	P.I. Plasticity Index	× 8
p Pocket	Penetrometer Resistance	w Moisture	e Content	L	.S. Linear Shrinka	ge

A	A.S.JAMES PTY. LTD.	Location: PROPOSED EASTERN Borehole: 9			
G	eotechnical Engineers		FOOTBAL	L HUB, WANTIRNA	
		Job No.	115706		Date: Mar '14
Soil Type	Description	Depth	Tests	Results	
FILL	Grey brown crushed rock Accessory Materials: Silt, sand	0 0.1 .	777		
FILL	Dry, medium dense Yellow brown / orange brown / grey mottled red stiff clay / silt Accessory Materials: Gravels Dry to moist, medium dense Note: tending to clay Likely Reworked Natural Material				
FILL	Dark grey / orange brown firm clay Accessory Materials: Silty Dry to moist, medium dense Likely Reworked Natural Material	1.6 .			
FILL	Dark brown / orange firm clay Accessory Materials: Silty Moist, medium dense, Note: Auger grabbing Possibly Reworked Natural Material	2.3 . 2.6 .			
FILL / CLAY (CH/CL)	Dark brown / orange / grey stiff clay Accessory Materials: Silty Moist, medium dense Passibly Beworked Natural Material	3.0		N - 3/5/6	
CLAY (CH/CL)	Grey / orange brown Silty Moist, stiff Dargile Formation BOREHOLE TERMINATED	3.4 .		11-07070	
+ Standard	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohesion	L.L. Liquid Limit	
I Undistur	bed Sample - Diameter Stated	Ø Friction	Angle	P.L. Plastic Limit	Figure
s Vane Sl	hear Strength Penetrometer Resistance	P Wet Der	isity Content	P.I. Plasticity Index	9

F	A.S.JAMES PTY. LTD.	Location:	PRO	POSE	D EASTERN	Borehole: 10
G	eotechnical Engineers		FOO	TBALL	- HUB, WANTIRNA	Data Marita
		Job No. Ground W	115/06 ator: N	6 		Date: Mar '14
Soil Type	Description	Depth		Tests	Results	
FILL	Grey brown crushed rock	0				
FILL	Dry, medium dense Orange brown / grey stiff clay	0.05 .				
	Accessory Materials: Gravels, silty Dry to moist, medium dense, Likely Reworked	1.0				
FILL	Brown gravels / silt Accessory Materials: Clay, plastic, brick fragments, glass, steel Dry to moist, medium dense Historic I andfill (1.0-1.4 m)	1.4 . 1.6 .		+	N = 3 / 3 / 4	
SILT	Pale grev	1 .				
(MH)	Clayey	2.5 .				
CLAY (CH/CL)	Moist, loose to medium dense (1.4-1.6 m) Dark Brown mottled orange /grey Silty Moist, stiff (1.6-2.5 m)	· ··· · ·		+	N = 2 / 3 / 4	
CLAY (CH/CL)	Grey / orange brown Silty Moist, stiff Ironstones (Sub-angular) present and deeper depths including occasional grit and gravel bands, variably ferruginous	· · · · ·		+	N = 4 / 6 / 7	
	BOREHOLE TERMINATED	6.4		+	N = 5 / 9 / 10	
		· · · · · · · · · · · · · · · · · · ·				
		· · ·				
		· ·				
+ Standar	d Penetration Test - N blows/150mm_incr	c Annaren	t Cohesio	n	iauid imit	
	rbed Sample - Diameter Stated	Ø Friction	Anale		P Plastic Limit	Figure
s Vane S	hear Strength	P Wet Der	nsity		P Plasticity Index	10
p Pocket	Penetrometer Resistance	w Moisture	e Content		L.S. Linear Shrinkag	e

F	A.S.JAMES PTY. LTD.	Location:	PRC	OPOSE	DEASTERN E	Borehole: 11
G	eotechnical Engineers		FOOTBALL HUB, WANTIRNA			
		Job No.	11570	06		Date: Mar '14
Soil Type	Description	Depth	ater:	Tests	Results	
FILL	Grey brown crushed rock	0		10313	ricouito	
	Accessory Materials: Silt, sand Dry, medium dense (0-0.05 m)	0.05 . 0.3 .	\square			
FILL	Accessory Materials: Silty Dry, medium dense, (0.05-0.3 m)	· ·				
FILL	Orange brown / grey stiff clay Accessory Materials: Gravels, silty Dry to moist, medium dense, Likely Reworked	1.6 .				
	Natural Material (Capping Layer) (0.3-1.6 m)	•				
FILL	Grey brown / orange brown / dark grey firm clay Accessory Materials: Silt, plastic, gravels, glass Moist, loose to medium dense Historic Landfill (1.6-3.5 m)					
	Note: Silty Clay lenses	· · · · · · · · · · · · · · · · · · ·				
CLAY (CH/CL)	Orange brown /grey Silty Moist, stiff	3.5 .				
CLAY (CH/CL)	Grey / orange brown Silty, tending sandy Moist, stiff Ironstone (Sub-angular) gravels present at deeper depths including occasional grit and gravel	5.0 5.0		+	N = 4 / 6 / 7	
	bands, variably ferruginous Dargile Formation BOREHOLE TERMINATED	 6.4 .		+	N = 6 / 7 / 9	
		· · ·				
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohes	ion	L.L. Liquid Limit	
I Undistu	rbed Sample - Diameter Stated	Ø Friction	Angle		P.L. Plastic Limit	Figure
s Vane S	hear Strength	P Wet Der	nsity		P.I. Plasticity Index	11
p Pocket	Penetrometer Resistance	w Moisture	e Conter	nt	L.S. Linear Shrinkage	

A	A.S.JAMES PTY. LTD.	Location:	PROPO	OSED	EASTERN	Borehole: 12
G	eotechnical Engineers		FOOTE	BALL	HUB, WANTIRNA	Data Marita
		Job No.	115/06			Date: Mar '14
Soil Type	Description	Depth		sts	Results	
FILL	Grey brown crushed rock Accessory Materials: Silt, sand Dry, medium dense (0-0.05 m)	0 0.05 .				
FILL	Orange brown / grey stiff clay Accessory Materials: Gravels, silty Dry to moist, medium dense, Likely Reworked Natural Material (Capping Layer)	· 				
FILL	Dark Grey brown firm clay Accessory Materials: Silt, plastic, gravels, sand Moist, loose to medium dense Historic Landfill					
CLAY (CH/CL)	Grey / orange brown Silty Moist, stiff Ironstone (Sub-angular) gravels present at deeper depths including occasional grit and gravel bands, variably ferruginous Dargile Formation	4.0 4.0		+ N	l = 4 / 6 / 8 l = 11 / 13 / 14	
	BOREHOLE TERMINATED					
+ Standard I Undistur s Vane S p Pocket	d Penetration Test - N blows/150mm. incr. rbed Sample - Diameter Stated hear Strength Penetrometer Resistance	c Apparent Ø Friction P Wet Den w Moisture	Cohesion Angle sity Content		L.L. Liquid Limit P.L. Plastic Limit P.I. Plasticity Index L.S. Linear Shrinkag	Figure 12

A	A.S.JAMES PTY. LTD.	Location:	PROPO	SED EASTERN	Borehole: 13
G	eotechnical Engineers		FOOTB/	ALL HUB, WANTIRNA	N
		Job No.	115706		Date: Mar '14
Soil Type	Description	Depth		ts Results	
FILL	Grey brown crushed rock Accessory Materials: Silt, sand Dry, medium dense (0-0.05 m)	0 0.05 .	777		
FILL	Orange brown / red / grey stiff clay Accessory Materials: Trace gravels, silty Dry to moist, medium dense, Likely Reworked Natural Material	1.6 .			
FILL / CLAY (CL / CH)	Orange brown /grey Silty Moist, stiff Possibly Reworked Natural Material	2.5 .	+	N = 7 / 9 / 12	
CLAY (CH/CL)	Grey / orange brown Silty, tending sandy Moist, stiff Ironstone (Sub-angular) gravels present at deeper depths including occasional grit and gravel bands, variably ferruginous Dargile Formation	· · · · · ·	+	N = 7 / 9 / 10 N = 9 / 12 / 22	
	BOREHOLE TERMINATED	6.4 .			
+ Standard I Undistur s Vane Si p Pocket	d Penetration Test - N blows/150mm. incr. bed Sample - Diameter Stated hear Strength Penetrometer Resistance	c Apparent Ø Friction A P Wet Den w Moisture	Cohesion Angle sity Content	L.L. Liquid Limit P.L. Plastic Limit P.I. Plasticity Index L.S. Linear Shrinkag	Figure 13

A.S.JAMES PTY. LTD.		Location:	Location: PROPOSED EASTERN Borehole: 14				
G	eotechnical Engineers		FOC	OTBALL	- HUB, WANTIRNA		
		Job No.	11570	06	d and the state	Date: Mar '14	
Soil Type	Description	Ground Wa	ater:	Percne	d water table at app	roximately 3.0 m.	
Soli Type	Orange brown silt	0		16212	nesuits		
	Accessory Materials: Gravels , clayey		777				
	Dry to moist, medium dense		///				
	Likely Reworked Natural Material						
	Note. Tending to sity day		///				
			///				
		•					
			$\langle / / \rangle$				
			///				
			//				
		•					
FILL	Orange brown / grey stiff clay	1.4 .	///				
	Accessory Materials: Gravels , silty		$\overline{//}$				
	Dry to moist, medium dense						
	(Capping Layer)	1.8 .	$\langle / / \rangle$				
			///				
			//				
FILL	Grey brown firm clay Accessory Materials: Silt plastic glass steel	•					
	sand, sole of shoe		V//				
	Moist, loose to medium dense		//				
	Historic Landfill						
			V//				
			V//				
		•					
			///				
			///				
			$\langle / / \rangle$				
		•	$\langle / / \rangle$				
			///				
			//				
		•					
		4.5	V//				
CLAY	Orange /grey	4.6 .					
(CH / CL)	Silty						
	Dargile Formation						
	BOREHOLE TERMINATED						
		· ·					
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohesi	ion	L.L. Liquid Limit		
I Undistur	rbed Sample - Diameter Stated	Ø Friction	Angle		P.L. Plastic Limit	Figure	
s Vane S	hear Strength	P Wet De	nsity		P.I. Plasticity Index	14	
р Роскет	reneuonneler Resistance	w ivioisture	+ Conter	n.	L.S. Linear Shrinkage	5	

A	A.S.JAMES PTY. LTD.	Location:	PROPOSE	ED EASTERN B	orehole: 15 & 16	
G	Geotechnical Engineers		FOOTBAL	L HUB, WANTIRNA		
		Job No.	115706		Date: Mar '14	
Soil Type	Description	Depth	ater. NIL			
BH15 FILL	Grass Playing Surface Brown Sand Accessory Materials: silt, organics Moist, Medium Dense (0 - 0.1 m)	0 0.1 .				
FILL	Brown Sand / Silt / Gravels Moist, Medium Dense (0.1 - 0.3 m)	0.3 . 		s > 140 kPa		
CLAY (CH / CL)	Orange / grey / red Silty Moist, very stiff Dargile Formation	· · · ·		s > 140 kPa		
				s = 120 kPa		
	BOREHOLE TERMINATED	2.0				
BH16 FILL FILL	Grass Playing Surface Brown Sand Accessory Materials: silt, organics Moist, Medium Dense (0 - 0.1 m) Brown Sand / Silt / Gravels	0 0.1 . 0.3 .				
FILL	Moist, Medium Dense (0.1 - 0.3 m) Orange brown / yellow brown silt Accessory Materials: Clay Dry to moist, medium dense, Likely Reworked Natural Material			a 114 kBa		
CLAY (CH / CL)	Orange /grey Silty Moist, very stiff Dargile Formation			s = 136 kPa		
	BOREHOLE TERMINATED	2.0 2.0				
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparent	Cohesion	L.L. Liquid Limit		
I Undistu	rbed Sample - Diameter Stated	Ø Friction	Angle	P.L. Plastic Limit	Figure	
s Vane Shear Strength p Pocket Penetrometer Resistance		P Wet Der w Moisture	sity Content	P.I. Plasticity Index L.S. Linear Shrinkage	10	

A	A.S.JAMES PTY. LTD.	Location:	PRO	POSE	D EASTERN	orehole: 17 & 18
G	eotechnical Engineers		FOO	TBAL	L HUB, WANTIRNA	
		Job No.	11570	6		Date: Mar '14
	Description	Ground W	ater: N		Deculto	
Soli Type	Description Grass Blaving Surface	Depth		lests	Results	
FILL	Brown Sand	0				
	Accessory Materials: silt, organics	0.1 .	777			
	Moist, Medium Dense (0 - 0.1 m)		///			
FILL	Brown Sand / Silt / Gravels	0.3 .	$\langle / / \rangle$			
	Moist, Medium Dense (0.1 - 0.3 m)					
				•	s > 140 kPa	
01.437		•				
	Orange /grey					
	Moist verv stiff					
	Dargile Formation				s > 140 kPa	
	5					
		1.3 .				
SAND	Fine to Medoum Grained		11			
(SM)	Grey Brown / yellow brown mottled red		111	•	s > 140 kPa	
	Silty		111			
	Noisi		111			
			///			
	BOREHOLE TERMINATED	2.0	/// ////			
BH18	Grass Playing Surface					
FILL	Brown Sand	0				
	Accessory Materials: silt, organics	0.1 .				
	Moist, Medium Dense (0 - 0.1 m)					
FILL	Brown Sand / Silt / Gravels	0.3 .				
	Moist, Medium Dense (0.1 - 0.3 m)					
FILL	Orange brown / yellow brown silt					
	Accessory Materials: Clay Dry to moist medium dense Likely Reworked					
	Natural Material		$\sqrt{/}$			
			V/V			
			///			
			V/A			
o		1.3 .	K//			
	Orange / grey brown mottled red				a 102 kBa	
	Moist stiff			•	5 = 102 KFa	
(0117 02)	Note: Possibly reworked Natural Materials					
	BOREHOLE TERMINATED	2.0	μщ			
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohesio	on	L.L. Liquid Limit	
I Undistu	rbed Sample - Diameter Stated	Ø Friction	Angle		P.L. Plastic Limit	Figure
s Vane S	hear Strength	P Wet Der	nsity		P.I. Plasticity Index	16
p Pocket Penetrometer Resistance		w Moisture	e Content	t	L.S. Linear Shrinkage	

A.S.JAMES PTY. LTD.		Location: PROPOSED EASTERN Borehole: 19 & 20				
G	eotechnical Engineers		FO	OTBAL	L HUB, WANTIRNA	
		Job No.	1157	06		Date: Mar '14
	Description	Ground W	ater:	NIL	Desults	
Soli туре вная	Grass Plaving Surface	Depth		Tests	Results	
FILL	Brown Sand	0				
	Accessory Materials: silt, organics	0.1 .	777			
	Moist, Medium Dense (0 - 0.1 m)					
SILT	Grey brown containing trace gravels and sand	0.3 .				
(ML)	Moist, Medium Dense (0.1 - 0.3 m)	•				
				•	s > 140 kPa	
CLAY	Grev Brown / orange brown / red at deeper depths					
(CH / CL)	Silty					
	Moist, very stiff					
	Dargile Formation			•	s = 130 kPa	
					s = 140 kPa	
		20				
		2.0				
BH20	Grass Playing Surface					
FILL	Brown Sand	0				
	Accessory Materials: sill, organics	0.1.	++-			
FILL	Brown Sand / Silt / Gravels	0.3.	///			
	Moist, Medium Dense (0.1 - 0.3 m)		///			
FILL	Orange brown / grey brown stiff clay		///			
	Accessory Materials: Silt		$\langle / / \rangle$			
	Dry to moist, medium dense, Likely Reworked		///			
	Natural Material	•				
		1.0	///		s = 94 kPa	
			Π́Π	-		
CLAY	Grey Brown / orange brown					
(CH / CL)	Silty				00 / D	
	Moist, very stiff			•	s = 90 kPa	
	Dargne Formation	•				
	BOREHOLE TERMINATED	2.0	ЩЩ			
		•				
		•				
+ Standar	d Penetration Test - N blows/150mm. incr.	c Apparen	t Cohes	sion	L.L. Liquid Limit	
I Undistu	rbed Sample - Diameter Stated	Ø Friction	Angle		P.L. Plastic Limit	Figure
s Vane S	hear Strength	P Wet Der	nsity		P.I. Plasticity Index	17
p Pocket Penetrometer Resistance		w Moisture	e Conte	nt	L.S. Linear Shrinkage	



DRAFT



Site Visits, Landfill Gas and Personal Recounts

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ANZ

ΑΞϹΟΜ

FQM - Landfill Services and Utilities Surveys Data Sheet

Q4AN(EV)-702-FM1

Project Name:	Wantirna Reserve		Project Number:	60548185
Project Location:	Wantirna Reserve		Client:	ксс
PM Name:	WD		Date:	7/08/2019
Monitoring Instrument and Serial Number:	Inspectra Las	er - 1215	Fieldwork Staff:	VL
			Field Data	
Point ID	Easting	Northing	CH4 (ppmv)	Comments/ Features
Ambient Air			1.3-1.5	Taken in Carpark
10			1.8	Carpark Drain
9			2-2.4	Carpark Drain
17			1.6	Entrance Road - pit
32			1.6	Entrance Road - pit
33			4.6	Entrance Road - pit
6			1.5-1.6	Telstra Pit
7			3.9	Drain
18			1.6	Pole
20			1.6	Flag Pole
34			1.5	Utility Pit
29			1.6	Building - Scout Hall underfloor vents
22			1.7	Pole
5			1.7	Triangular Drain - East
3			1.7	Triangular Drain - West
35			1.8	Electrical Box
16			1.6	Utility Pit/Drain
21			1.9	Light Pole
31			1.6-1.8	Building - Cricket Pavillion underfloor vents, drainpipes, etc
8			1.7	Stormwater Drain
4			1.7	Utility Pit
1			1.8	Drain
2			1.6	Drain
36			1.4	Sign Post
12			1.5	Drain
28			1.1	Sign Post
27			0.7	Sign Post
19			0.7	Water Tap
23			0.7	Sign Post
26	1		0.8	Sign Post
37			1.4	Old Oval Pole
25			1.3	Sign Post
13			1.4	Drain
14	1		1.4	Drain

Monitoring Instrument and Serial Number:

Project Name: Project Location: PM Name:

FQM - Landfill Cap Surveys Data Sheet

Wantirna Reserve		Project Number:	60548185		
Wantirna Reserve		Client:	ксс		
WD		Date:	7/08/2019		
Inspectra Laser - 1215		Fieldwork Staff:	VL		
Field Data					
Eacting	Northing	CH4			

Point ID	Easting	Northing	CH4 (ppmv)	Comments/ Features
Ambient Air			1.3-1.5	Taken in Carpark
Carpark Western Posts			1.3-1.4	Exposed posts
Carpark West of Drains			1.2-1.5	Exposed posts
Carpark Eastern Posts			1.2-1.6	Exposed posts
Roundabout Posts			1.3-1.5	
Concrete Slab			1.5	
Roundabout SW Gate			1.3	
SW Drain with trench			1.5	
Roundabout South Posts			1.3-1.6	
Roundabout South Track			1.4-1.6	
Roundabout Southeast Subsidence in Asphalt			1.7-1.8	
Roundabout inner circle and adjacent Cracks in Asphalt			1.5-1.6	
Entrance road poles along western boundary			1.5-1.6	
Tennis Club Carpark Drainage Pit			1.6	
Tennis Club Drain under entrance			1.6	
Tennis Club Western Side Fence			1.5-1.6	
Main Roundabout sign poles			1.4-1.6	
Posts on road to Scout Hall			1.5-1.6	
Scout Hall Carpark Pole			1.5	Opposite Playground
Northwest Carpark Posts			1.6-1.8	
Short yellowing grass South of NW Carpark			1.6-1.8	May be due to short mowing
Cricket Cages			1.6-1.9	Posts and cracks in concrete and concrete slab edges
Park Sign near cages			1.6	
Concrete Path edge north of Batting Cages			1.6-1.9	
Posts near 2 and 12			1.5	
Cut Grass walkway between 19 and 11 in grassed area north of carpark			0.8-1.4	
Gravel in carpark			1.3	
Path east west direction north of			1.2-1.3	

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Q4AN(EV)-702-FM1

AECOM

Equipment Calibration Check Inspectra Laser



Enqip #:	9718
Company:	AECOM Australia Pty Ltd
Consultant:	Vera Levina
PO #:	60548185 task 2.2
Certificate #:	13594

	UNI	IT IDENTIFICATION
Model Number:	Inspectra Laser	
Serial Number:	CH48921215	
Unit Type:	Methane Analyser	
	INSPECTION R	ECORD/CONDITION REPORT
low Rate:	PASS	LOOKD/CONDITION REPORT

Alarms: PASS

	CALIBRATION	DETAILS	
Gas	Readin	ıg	Traceability Lot #
Nitrogen UHP	1.3	Dom	
Methane 100 ppm			1068301
	96.6 (ppm	1009971
Methane 2.5 %	2.4	%	977870

Calibration Successful: YES

Calibrated By:

Doyle Schapendonk

Test Date:

6/08/2019

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nterview Questions – Wantima Réserve late:	AECOM	Imagine Deliver
What were the stormwater pond and private ponds as marked on the ma	ips used for?	
potentially for orchards area.		
Redevelopment of tip into oval in ~1975– when and by who?		_
What happened during construction of additional tennis courts #6-10 - so Fill brought in for development - 6+5 9-10 were in northern corner. (+ 9 settlement but found o	tions of netfonce not	
Could you outline on the map where you think waste was placed and extended?	ent of landfilled area north of te	ennis
Other comments		_
Background: Alison lived in avea from 1980 - p time in friends: properties here prior. Remembers th some development of the oval after.	ivescut, but did spend is till being there and	
MF: council auscussions with roundabout acvelopers f	iom 1980 - encountered	1

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From:	Marissa France <marissa.france@knox.vic.gov.au></marissa.france@knox.vic.gov.au>
Sent:	Wednesday, 24 July 2019 10:23 AM
То:	McCartney, Breana
Cc:	Levina, Vera
Subject:	RE: Wantirna Reserve information

Hi Breana,

Nice to meet you too (finally)!

So I have found a font of knowledge on the waste collection front, one of our contractors Richard Lever. He has advised that in Knox from pre 1950s to approx. 1968 waste collection was carried by a contractor (Cook), and you use to put the bin out with a coin under the bin (as a way to pay for the service). In 1968 a company called Vantiller and Scott collected in the area until approx. 1975 when Cleanaway started. As the area was mostly orchards there was not a large domestic collection service. Richard said he thought the name of the access road to the old tip was Koomba Road, and that from about 1975 onwards people did refer to the area as the old tip but there definitely was signage on the old access road in the mid 1970s. (He does not recall the tip being there as it was before his time, but that people did definitely refer to it as the old tip and that Vantiller did tip waste there).

Thanks

Marissa France | Waste Management Engineer +61 3 9298 8234

511 Burwood Highway Wantirna South Victoria 3152 Knox.vic.gov.au



1

From: Marissa France <<u>Marissa.France@knox.vic.gov.au</u>> Sent: Tuesday, 23 July 2019 4:43 PM To: McCartney, Breana <<u>Breana.McCartney@aecom.com</u>> Cc: Levina, Vera <<u>vera.levina@aecom.com</u>> Subject: Wantirna Reserve information

Hi Breana,

Thank you for meeting with us out on-site yesterday.

As a follow up I have been provided with, and attempted to make contact with some members from the football club and cricket clubs. Once I make contact I will provide you with responses.

Also as we had discussed there is a member of staff here who worked on the installation of the access roads and carparks at Wantirna Reserve. He recalls that the internal road waste OK, up until the roundabout. He said once they got to putting in the round about they had excavated to about 0.5m depth and found thousands of rubber fan felts mixed with liquid. He said he cannot remember anything else.

I have uncovered a couple of other emails with some history .

Attached is a plan dated 1965 that shows the existing oval just north of the first lot of tennis courts and a pavilion (as the pre 1970 aerials had shown). It also shows an open channel in the approx. location of where the round about may have ended up. (Map M605 (1)).

A contour map with existing site conditions in 1979

The oval and pavilion (in the location where the landfill was) became settled and uneven and ceased being used as a sporting field in 1989. It was converted by Parks Vic to a picnic shelter and was later demolished due to significant movement. (Believe this aerial is from 1996).

2



Still searching for any more informtion.

Thanks Marissa France | Waste Management Engineer +61 3 9298 8234

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KNOX CITY COUNCIL

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McCartney, Breana

From:	Marissa France < Marissa.France@knox.vic.gov.au>
Sent:	Tuesday, 30 July 2019 12:17 PM
То:	McCartney, Breana; Levina, Vera
Subject:	Information - Wantirna Reserve

Hi Breana and Vera

Just spoke with a man called Bernie (Whitney?) who has played cricket at the site since the late 1960's, and before I forget here is a dot point of his recollections:

- Original cricket ground in the 'pre 1970 field location'
- A landfill was located just to the north of that ground (in the 1975-1991 oval location)
- On hot days there was a strong smell from the landfill •
- It was a Council tip, but people could drive in with trailers if they wanted. It was fairly open. ٠
- The oval then moved to the area where the landfill was. Council was going to put turf on the oval, but members didn't want it so the ovals were top dressed
- If any work was done on the ground they would be pulling out paper and rubbish •
- The ground ended up settling 'like a potato chip'
- Recalls stormwater pond to the NE of the oval •
- Does not recall the private pond further to the west •
- He recalls a deep guttering of the main drive (the drive to the tip), and there would be a lot of water in it when it rained.

He is going to pass my contact details onto another lady, and if she thinks she has any information of worth she will give me a call.

Thanks

Marissa France | Waste Management Engineer

+61 3 9298 8234

511 Burwood Highway				
Wantirna South	11			
Victoria 3152			C 2	- New
Knox.vic.gov.au		10/	N B	Knox City Council

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WANTIRNA TENNIS CLUB - COURTS & GROUNDS WORKS DETAILS FOR KNOX CITY COUNCIL MAY, 2019

BACKGROUND

At a meeting between Knox City Council and Wantirna Tennis Club representatives, it was decided that we would search our records and interview older members in an attempt to provide details including timelines of ground works (within the Tennis club's area) including:

- Dates when courts were installed,
- Dates when other major construction occurred including light towers etc.,
- Other significant developments occurred, and
- Identification of the (now disused) rubbish tip boundaries.

EXECUTIVE SUMMARY

- 1. Ground movements within the Tennis court grounds have been minimal for the past 40 plus years and explainable.
- 2. The previous rubbish tip was nowhere near the Tennis club for at least 60 plus years.

SUMMARY OF FINDINGS FOLLOWING DISCUSSIONS WITH MEMBERS AND A SEARCH OF OUR RECORDS.

- The club was established on the present site in 1934.
- One of our club members played at the club when she was 11 years old (63 years ago) and is willing to identify the site of the tip which was and as quoted "nowhere near the Tennis courts". There are also other older members that can be interviewed.

DEVELOPMENTS	COMMENTS – (re ground stability & evidence of a rubbish tip within tennis club perimeter)
1935 CLUB ESTABLISHED	The original courts, (Bitumen 1) and then sand 2 were located in the south east corner of the
	existing site. This is the site of courts 1, 2 & 3. (refer map) The ground has always been stable & no
	remedial works undertaken
Developments though to the	A search of records & member feedback indicates that not one of these courts have ever had a
late 1960's saw the progressive	"complete" rebuild, ie. only refurbishments have been undertaken, the last of which was
installation of 5 en tout cas	approximately 25 years ago. Hence evidence is that the ground is stable & we believe that there is
courts along Mountain	no reason to believe that the rubbish tip extended to this area. (Note: some of these courts are
Highway.	constructed using coke (ie. a coal derivative) as part of their drainage/foundation. As we know, coke
	has not been available in Melbourne since the early 1960's.

Construction of the present new club house in 1980	No evidence of building cracks or unstable ground exists
1975. 2 new courts (6 & 7) were constructed in north east corner of site	These courts have never been reconstructed nor refurbished. They have been maintained by club members/working bees. There have never been any ground movement issues.
In 1987 the existing court 8 was constructed.	This court has never been reconstructed nor refurbished. It has been maintained by club members/working bees. The only ground movement that has occurred (north eastern corner along the fence is believed to have been caused/influenced by inadequate precautions and ramifications during the construction of the sewerage line (to/from the Scout Hall) to the north of the boundary fence, and potentially the partial collapse of a surface drain near fence.
In 1993 Lights were installed on courts 1,6,7,8.	During this process, a member has identified that when the foundations were bored for the light tower on the northern boundary of court 8, rubbish/debris was encountered at approx. 2 meters deep. An inspection will reveal that this light tower & immediate area has been stable since then. No such debris was encountered when the foundations for the other light towers were bored. These courts were built over an area used as a car park, with considerable "fill" being added as part of the 'court construction processes. Court 10 is very stable has never been reconstructed nor refurbished. It has been maintained by club members/working bees and no ground movements/issues encountered. Court 9 however, has encountered, ground movement during/post the recent drought on the northern end. A visual inspection of the area will readily reveal: • At ground level there has been some (probable 120mm) sink age at both sides of the court. • The fence set in concrete foundations (approx.) .5 meter deep, has NOT sunk or moved. From these observations, we are of the opinion that the ground (court surface) "problems" in this immediate area (which we have not very successfully tried to rectify ourselves) were probably caused by settling of added fill during the court construction, or the collapse of installed court drainage and/or drought issues, &/or issues with the construction of the sewerage drain in approximately the late 1980's or early 1990's.
In 1994 Courts 9 & 10 were constructed.	

DARYL. Brief history. - Wantima Reserve was originally owned by J Biscoff & for Joseph Smithin 1858. Later Robert Triall, Berwick Concillor owned part of the holding on which the wanting Recreation Reserve now stands. The area was proclaimed a Reserve in 1925 and the Council provided land shortly after Wantina Club commenced in 1934 1959 First en tout cas courts installed (Sand previously) losting en tout cas 10 1952 3rd court built 1956 New pavilion build (not the fresent one) 1959 Court & build, 1961 New courts afened (Councillors attended) 1975 More courts built * April 1982 M.M.B. W (South least Water) announces take aver of lease for the area from Knox Council as of Ray 86. 1986 Court 8 built 1991 Renovations to Clublouse (Moving of Entry to clublouse) 1993 Court lighting to Courts 1, 6, 7, 78. 1994 Courts 9 + 10 built Built on old carfark

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1991 Pergola added to pavilion New playground installed 1993 Council \$ 5000-00 Club \$ 2000-00 Thanks to Councillors Hen Me Bride 4 Marie Wallace. Council Bruce Mc Cardney (Fracileties) John Daly & Fan Bell Borders and soft play installed by Club. 1994 Courts 9410 built. 1994 New carpark completed. 1993 Light sowers 6,7+8 installed 1993. New carpark started * Council supplied car park plans including landscaping plans and species of plants to be planted. Council sufflied sleeper edging and crushed rock Club supplied retaining wall rocks and plants for garden beds Club and also sufflied the machinery to build garden beds and retaining walls, including the speading of enshed rock the carfack. Club sufflied and fitted all signs and posts including centre ballards Club also obtained quotations for the pert + chanel and asphalting of carpark for Council.

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This carpark was to be only a sempory design until Council built an This is why they would ly do. design, and plans, and for the Club do all aker work so we could have a proper farking area, after our existing gravel carfark courts 94% now stand oosing 997 Major Clubhause extension and upgrade to so Tennis Victoria standard. 1997 20 x+? Rainwater lanks and sprinkler system. 494